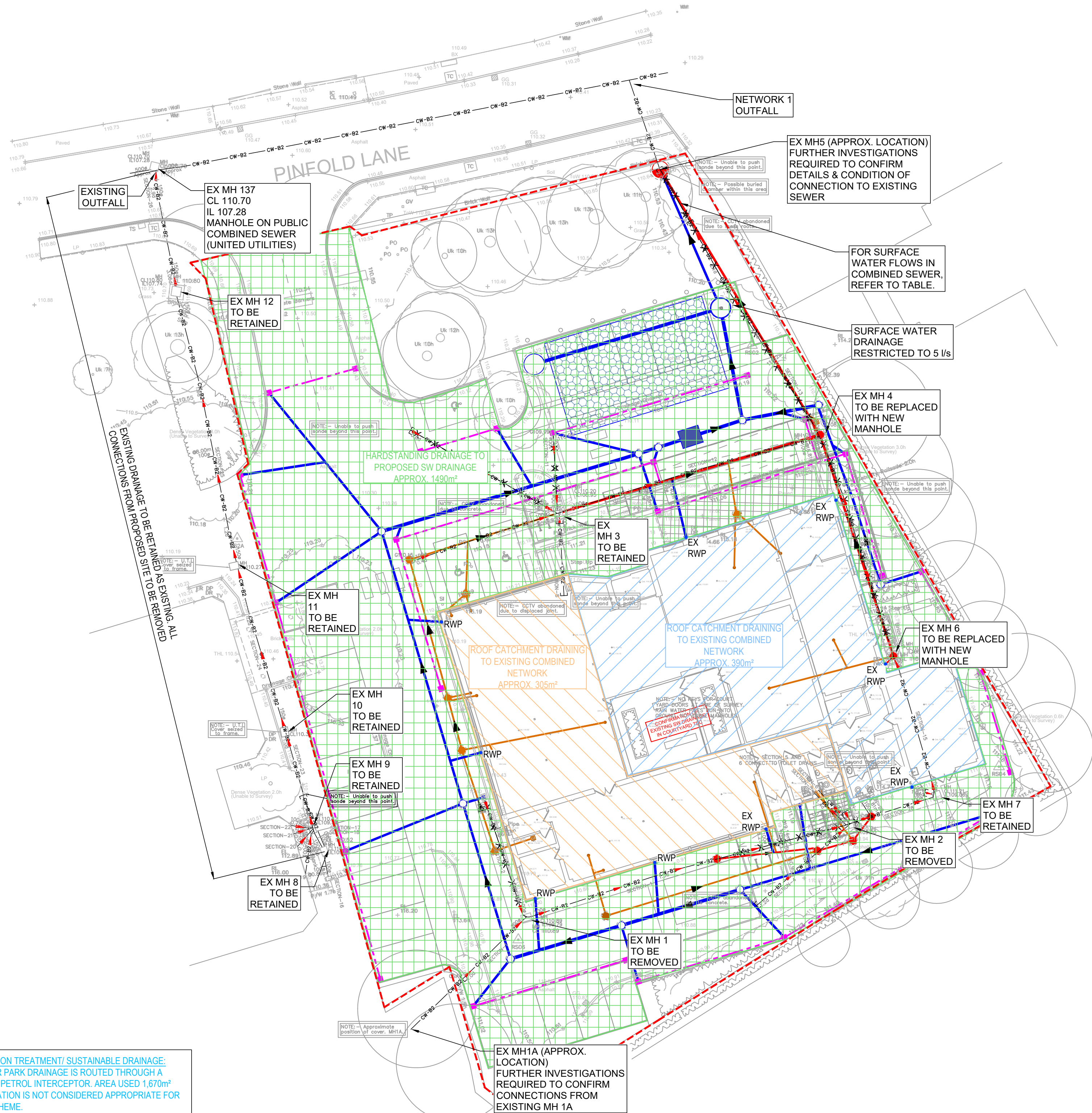




GENERAL NOTES:

1. THIS DRAWING IS TO BE READ IN CONJUNCTION WITH ALL RELEVANT ARCHITECTS AND ENGINEERS DRAWINGS AND SPECIFICATIONS.
2. DO NOT SCALE THIS DRAWING. ANY AMBIGUITIES, OMISSIONS AND ERRORS ON DRAWINGS SHALL BE BROUGHT TO THE ENGINEERS ATTENTION IMMEDIATELY. ALL DIMENSIONS MUST BE CHECKED / VERIFIED ON SITE.
3. DESIGN BASED ON TOPOGRAPHICAL SURVEY INFORMATION AVAILABLE AT THE TIME OF THE DESIGN.
4. ALL EXISTING SERVICES TO BE LOCATED PRIOR TO COMMENCEMENT OF ANY DRAINAGE WORKS, AND WHERE NECESSARY PROTECTION OR DIVERSIONS TO BE UNDERTAKEN TO AVOID CONFLICT WITH THE PROPOSED WORK.
5. OUTFALL CONNECTION(S) SUBJECT TO AGREEMENT WITH THE APPROVING AUTHORITY.
6. ALL DIMENSIONS ARE IN METRES AND LEVELS IN METRES ABOVE ORDNANCE DATUM UNLESS OTHERWISE NOTED.



- KEY:**
-  SITE BOUNDARY
 -  EXISTING PRIVATE COMBINED DRAINAGE
 -  EXISTING DRAINAGE TO BE REMOVED
 -  PROPOSED SURFACE WATER DRAINAGE
 -  PROPOSED FOUL DRAINAGE
 -  PROPOSED COMBINED DRAINAGE
 -  ROOF CATCHMENT DRAINING TO EXISTING COMBINED NETWORK
 -  ROOF CATCHMENT DRAINING TO PROPOSED SURFACE WATER NETWORK
 -  EXTERNAL CATCHMENT DRAINING TO PROPOSED SURFACE WATER NETWORK
- RWP • PROPOSED RWP
- EX • EXISTING RWP

EXISTING COMBINED DRAINAGE NETWORK PRE & POST-DEVELOPMENT PEAK FLOWS			
STORM EVENT	PRE-DEVELOPMENT PEAK FLOW RATE (l/s)	POST-DEVELOPMENT PEAK FLOW RATE (l/s)	FLOW REDUCTION
1-YEAR	15.9	4.6	71%
30-YEAR	25.1	11.1	56%
100-YEAR + 45% CC	28.6	17.1	40%

DRAWING BASED ON:

TOPOGRAPHICAL SURVEY
• 72968MCLS-01TOPOGRAPHICAL SURVEY 24.04.25

CCTV SURVEY

- S26150-U CCTV SURVEY BY SEP DATED MAY 25

GPR SURVEY

- 72968NGUG-01 GPR SURVEY BY SURVEY SOLUTIONS DATED MAY 25

ARCHITECT'S PLANS

- WFLREF-GDA-V1-S1-DR-A-1002_P06_PROPOSED SITE
- WFLREF-GDA-V1-00-DR-A-1001_P04_PROPOSED GROUND FLOOR GA PLAN
- WFLREF-GDA-V1-R1-DR-A-2001_P01_PROPOSED ROOF PLAN

STRUCTURAL PLANS

- 087132-CUR-XX-GF-D-S-20001 P01 GROUND FLOOR GENERAL ARRANGEMENT

DESIGN PHILOSOPHY

SURFACE WATER DESIGN
THE DESIGN PROVIDES CAPACITY UP TO THE 100 YEAR
45% CLIMATE CHANGE EVENT.
SW FLOW RATE: 5 l/s (FLOW CALCULATED BASED ON
GREENFIELD RUNOFF RATE, INCREASED TO ACHIEVE
MINIMUM ORIFICE SIZE TO PREVENT BLOCKAGES)

ATTENUATION
APPROXIMATE STORAGE REQUIREMENT-64m³ PROVIDED
WITHIN CELLULAR STORAGE.

OUTFALL:
SITE DISCHARGES TO THE EXISTING 500Ø COMBINED
SEWER IN PINFOLD LANE VIA AN EXISTING COMBINED
CONNECTION.
A \$106 AGREEMENT IS REQUIRED WITH UNITED UTILITIES.
PART 1 BY CURTINS
PART 2 ASSUMED NOT REQUIRED AS CONNECTION IS
INDIRECT.

LEVELS INDICATE THAT PUMPING WILL NOT BE REQUIRED.

POLLUTION TREATMENT/ SUSTAINABLE DRAINAGE:
THE CAR PARK DRAINAGE IS ROUTED THROUGH A BYPASS PETROL INTERCEPTOR. AREA USED 1,670m²
INFILTRATION IS NOT CONSIDERED APPROPRIATE FOR THIS SCHEME.

EXISTING RAINWATER PIPES
WHERE EXISTING RAINWATER PIPES ARE TO BE RETAINED, THESE WILL BE CONNECTED INTO THE AMENDED FOUL/ COMBINED NETWORK ON SITE. THIS IS TO REDUCE THE RISK OF FLOODING TO THE BUILDING. THIS ACCOUNTS FOR APPROXIMATELY 250m².

ALL NEW RAINWATER PIPES, WHICH SERVE APPROXIMATELY 290m² OF THE ROOF AREA, ARE CONNECTED TO THE NEW SURFACE WATER DRAINAGE NETWORK.

Curtins

Merchant Exchange, 17-19 Whitworth Street West, Manchester, M1 5WG
0161 236 2394
manchester@curtins.com
www.curtins.com

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Status:

STAGE 3

Project

WHITEFIELD MEDICAL CENTRE

Drug Title:

PROPOSED DRAINAGE PLAN

Drawn By

Designed By

	Date 29/04/2025	Scales @ A1 1:100
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087132 - CUR - XX - XX - DR - C - 92520

Revision					
P01					