

APPENDIX D
Existing Impermeable Areas Plan



- NOTES:**
1. THIS DRAWING IS INTENDED TO INDICATE THE EXISTING DRAINED AREAS FROM THE STAR RADCLIFFE ACADEMY DEVELOPMENT. IT IS BASED UPON A REVIEW OF THE TOPOGRAPHICAL SURVEY AND AERIAL IMAGES OF THE SITE.
 2. FOR THE TOPOGRAPHICAL SURVEY, REFER TO GREENHATCH DRAWING FS1013-GHG-XX-XX-D-U-0002.
 3. EXISTING DISCHARGE RATES GIVEN HAVE BEEN CALCULATED USING MICRODRAINAGE. BROWNFIELD RUN-OFF RATES ARE BASED ON THE HARD PAVED AREAS.

KEY

- PLANNING BOUNDARY
- DEVELOPMENT BOUNDARY
TOTAL AREA: 45620m² (4.562ha)
- SOFT LANDSCAPING AREA
TOTAL AREA: 25800m² (2.58ha)
- HARD LANDSCAPING AREA
TOTAL AREA: 19820m² (1.982ha)

GREENFIELD RUN-OFF RATES	
EVENT	GREENFIELD RUN-OFF
1:1	51.9 l/s
1:2	58.8 l/s
1:30	91.8 l/s
1:100	103.7 l/s
Q _{50%}	59.7 l/s

GREENFIELD RUN-OFF CALCULATED IN MICRODRAINAGE USING SOURCE CONTROL

P02	COMMENTS INCORPORATED	08.09.23	HW	PW
P01	STAGE 3 ISSUE	10.08.23	SA	PW
Rev.	Description:	Date:	By:	Chkd:



Renaissance Associates Ltd
Carvers Warehouse
77 Dale Street
Manchester
M1 2HG

Project:
STAR RADCLIFFE ACADEMY

Drawing Title:
EXISTING DRAINED AREAS

Status:
STAGE 3

Size:	Date:	Drawn by:	Designed by:	Checked by:
A1	JUN 23	HW	--	PW

Scale:	500	Project No:	2303-02
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Project:	Originator:	Volume:	Level:	Type:	Role:	Category/Number:	Rev:
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FS1013-REN-EX-00-D-C-5200-P02

APPENDIX E
InfoDrainage Greenfield Run-off Rates

UK and Ireland Rural Runoff Calculator

ICP SUDS / IH 124 ADAS 345 FEH ReFH2 Greenfield Volume

Method ☒ ICP SUDS ☐ IH 124

Area (ha)

SAAR (mm) Map

Soil

Region ...

Additional Options

Urban

Return Period (years)

Growth Curve

Calculate

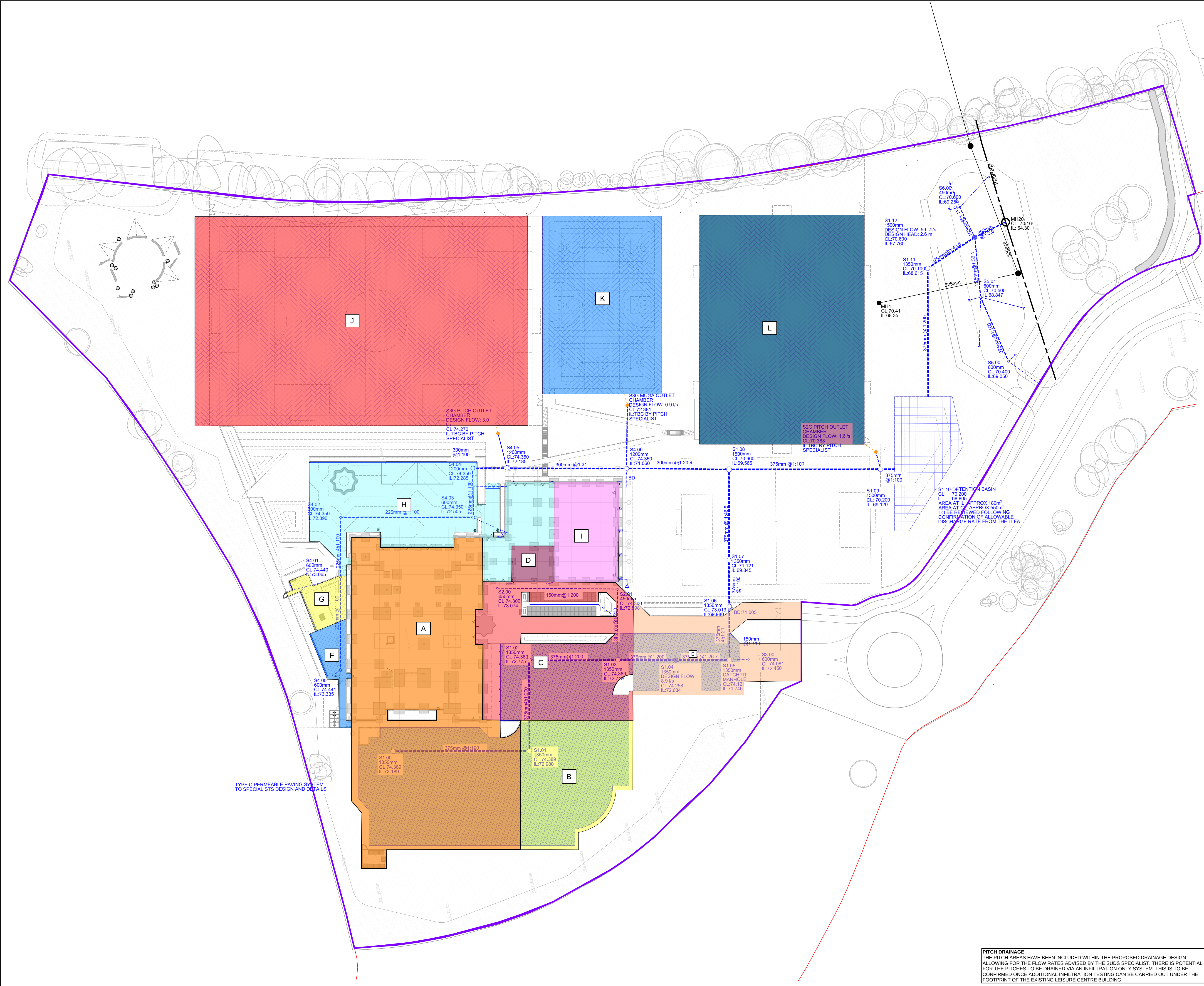
Results

Region	QBAR Rural (L/s)	QBAR Urban (L/s)	Q 2 (years) (L/s)	Q 1 (years) (L/s)	Q 30 (years) (L/s)	Q 100 (years) (L/s)
Region 10	34.8	59.7	58.8	51.9	91.8	103.7

OK Cancel

Help

APPENDIX F
Proposed Impermeable Areas Plan



NOTES:
1. THIS DRAWING SHOULD BE READ IN CONJUNCTION WITH
-EXISTING IMPERMEABLE AREA PLAN
-PROPOSED BELOW GROUND DRAINAGE DRAWING
-MICRODRAINAGE MODEL AND CALCULATION OUTPUTS

PLANNING BOUNDARY
DEVELOPMENT BOUNDARY

PROPOSED IMPERMEABLE AREA SUMMARY		
MANHOLE	HATCH	AREA (ha)
S1.00	A	0.374
S1.01	B	0.105
S1.02	C	0.154
S2.00	D	0.013
S1.03	E	0.102
S4.00	F	0.016
S4.01	G	0.017
S4.04	H	0.161
S4.06	I	0.055
S3G PITCH	J	0.585
S3G MUGA	K	0.174
S2G PITCH	L	0.320
S5.00	M	0.028
S5.01	N	0.063
S6.00	O	0.012
TOTAL		2.179

SITEWIDE TOTAL AREA	
HATCH	AREA (ha)
IMPERMEABLE AREA	2.182
PERMEABLE AREA	2.380
TOTAL	4.562

P03	STAGE 4	08.12.23	SA	PW
P02	ISSUED FOR PLANNING	07.09.23	HW	PW
P01	STAGE 3	10.08.23	SA	PW
Rev:	Description:	Date:	By:	Chkd:

Renaissance Associates Ltd
Carvers Warehouse
77 Dale Street
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Project:
STAR RADCLIFFE ACADEMY, BURY

Drawing Title:
PROPOSED DRAINED AREAS PLAN

Status:
STAGE 4

Size:	Date:	Drawn by:	Designed by:	Checked by:
A1	AUG'23	SA	SA	PW
Scale:	1:500	Project No:	2303-02	

Project - Originator - Volume - Level - Type - Role - Number: Revision:

FS1013-REN-EX-00-D-C-5201 **P03**

PITCH DRAINAGE
THE PITCH AREAS HAVE BEEN INCLUDED WITHIN THE PROPOSED DRAINAGE DESIGN ALLOWING FOR THE FLOW RATES ADVISED BY THE SUDS SPECIALIST. THERE IS POTENTIAL FOR THE PITCHES TO BE DRAINED VIA AN INFILTRATION ONLY SYSTEM. THIS IS TO BE CONFIRMED ONCE ADDITIONAL INFILTRATION TESTING CAN BE CARRIED OUT UNDER THE FOOTPRINT OF THE EXISTING LEISURE CENTRE BUILDING.

APPENDIX G
InfoDrainage Quick Storage Estimate

Quick Storage Estimate

Input

Input Type

User Input

Area (ha)

2.189

Volumetric Runoff Coefficient

1.000

Discharge Rate (L/s)

59.7

Infiltration Rate (m/hr)

0.0

Safety Factor

2.0

Quick

Calculate

Create New

From Library

All

FSR

...

Method

FSR

Number of Storms

38

Max. Run Time (mins)

20160

Input

Results

2D Graph

Create New

Help

Quick Storage Estimate

Results

Quick Storage Estimate variables require approximate storage of between 993m³ - 1503m³.

These values are estimates only and should not be used for final design purposes.

Input

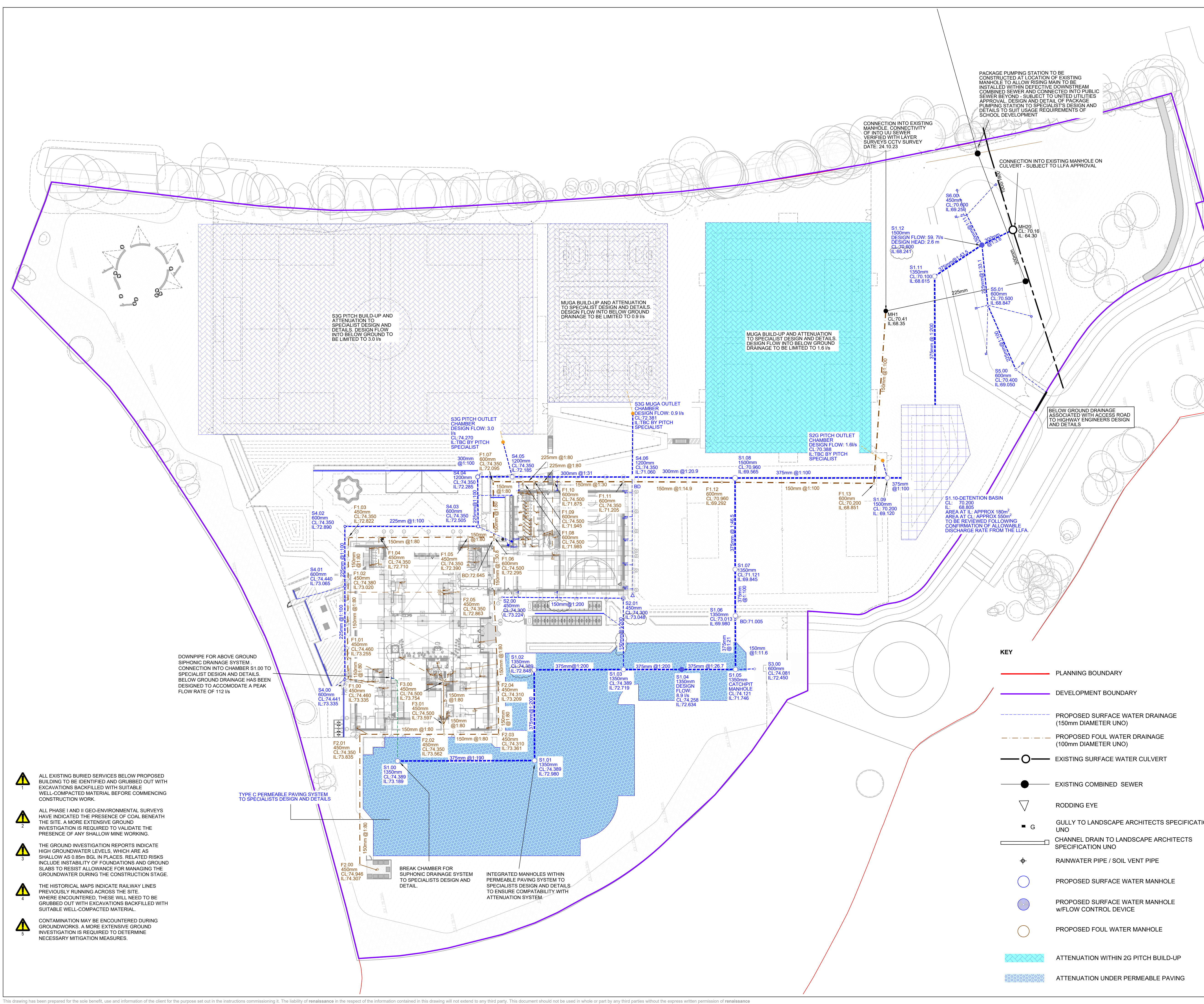
Results

2D Graph

Create New

Help

APPENDIX H
Below Ground Drainage GA



- 1 ALL EXISTING BURIED SERVICES BELOW PROPOSED BUILDING TO BE IDENTIFIED AND GRUBBED OUT WITH EXCAVATIONS BACKFILLED WITH SUITABLE WELL-COMPACTED MATERIAL BEFORE COMMENCING CONSTRUCTION WORK.
- 2 ALL PHASE I AND II GEO-ENVIRONMENTAL SURVEYS HAVE INDICATED THE PRESENCE OF COAL BENEATH THE SITE. A MORE EXTENSIVE GROUND INVESTIGATION IS REQUIRED TO VALIDATE THE PRESENCE OF ANY SHALLOW MINE WORKING.
- 3 THE GROUND INVESTIGATION REPORTS INDICATE HIGH GROUNDWATER LEVELS, WHICH ARE AS SHALLOW AS 0.85m BGL IN PLACES. RELATED RISKS INCLUDE INSTABILITY OF FOUNDATIONS AND GROUND SLABS TO RESIST ALLOWANCE FOR MANAGING THE GROUNDWATER DURING THE CONSTRUCTION STAGE.
- 4 THE HISTORICAL MAPS INDICATE RAILWAY LINES PREVIOUSLY RUNNING ACROSS THE SITE. WHERE ENCOUNTERED, THESE WILL NEED TO BE GRUBBED OUT WITH EXCAVATIONS BACKFILLED WITH SUITABLE WELL-COMPACTED MATERIAL.
- 5 CONTAMINATION MAY BE ENCOUNTERED DURING GROUNDWORKS. A MORE EXTENSIVE GROUND INVESTIGATION IS REQUIRED TO DETERMINE NECESSARY MITIGATION MEASURES.

TYPE C PERMEABLE PAVING SYSTEM TO SPECIALISTS DESIGN AND DETAILS

BREAK CHAMBER FOR SURPHONIC DRAINAGE SYSTEM TO SPECIALISTS DESIGN AND DETAIL.

INTEGRATED MANHOLES WITHIN PERMEABLE PAVING SYSTEM TO SPECIALISTS DESIGN AND DETAILS TO ENSURE COMPATABILITY WITH ATTENUATION SYSTEM.

CONNECTION INTO EXISTING MANHOLE. CONNECTIVITY OF INTO UL SEWER VERIFIED WITH LAYER SURVEYS CCTV SURVEY DATE: 24.10.23

PACKAGE PUMPING STATION TO BE CONSTRUCTED AT LOCATION OF EXISTING MANHOLE TO ALLOW RISING MAIN TO BE INSTALLED WITHIN DEFECTIVE DOWNSTREAM COMBINED SEWER AND CONNECTED INTO PUBLIC SEWER BEYOND - SUBJECT TO UNITED UTILITIES APPROVAL. DESIGN AND DETAIL OF PACKAGE PUMPING STATION TO SPECIALISTS DESIGN AND DETAILS TO SUIT USAGE REQUIREMENTS OF SCHOOL DEVELOPMENT

CONNECTION INTO EXISTING MANHOLE ON CULVERT - SUBJECT TO LLFA APPROVAL

MUGA BUILD-UP AND ATTENUATION TO SPECIALIST DESIGN AND DETAILS. DESIGN FLOW INTO BELOW GROUND DRAINAGE TO BE LIMITED TO 0.9 l/s

MUGA BUILD-UP AND ATTENUATION TO SPECIALIST DESIGN AND DETAILS. DESIGN FLOW INTO BELOW GROUND DRAINAGE TO BE LIMITED TO 1.6 l/s

BELOW GROUND DRAINAGE ASSOCIATED WITH ACCESS ROAD TO HIGHWAY ENGINEERS DESIGN AND DETAILS

KEY

PLANNING BOUNDARY

DEVELOPMENT BOUNDARY

PROPOSED SURFACE WATER DRAINAGE (150mm DIAMETER UNO)

PROPOSED FOUL WATER DRAINAGE (100mm DIAMETER UNO)

EXISTING SURFACE WATER CULVERT

EXISTING COMBINED SEWER

RODDING EYE

GULLY TO LANDSCAPE ARCHITECTS SPECIFICATION UNO

CHANNEL DRAIN TO LANDSCAPE ARCHITECTS SPECIFICATION UNO

RAINWATER PIPE / SOIL VENT PIPE

PROPOSED SURFACE WATER MANHOLE

PROPOSED SURFACE WATER MANHOLE w/FLOW CONTROL DEVICE

PROPOSED FOUL WATER MANHOLE

ATTENUATION WITHIN 2G PITCH BUILD-UP

ATTENUATION UNDER PERMEABLE PAVING

NOTES:

1. THIS DRAWING IS TO BE READ IN CONJUNCTION WITH ALL RENAISSANCE, ARCHITECTS, LANDSCAPE ARCHITECTS AND BUILDING SERVICES ENGINEERS DRAWINGS.

2. THIS DRAWING IS TO BE READ IN CONJUNCTION WITH THE BELOW GROUND DRAINAGE DETAILS SHOWN ON DRAWING SERIES C-5301 TO C-5304.

3. FOR ALL ABOVE GROUND DRAINAGE AND ASSOCIATED PUMPS REFER TO BUILDING SERVICES AND ARCHITECT'S DRAWINGS.

4. ALL BELOW GROUND DRAINAGE WORKS TO BE IN ACCORDANCE WITH RELEVANT BUILDING REGULATIONS AND SEWER SECTOR GUIDANCE APPENDIX C- DESIGN AND CONSTRUCTION GUIDANCE. IF DISCREPANCIES ARE NOTED BETWEEN SSG AND DRAWINGS THESE SHOULD BE BROUGHT TO THE ATTENTION OF RENAISSANCE FOR REVIEW.

5. RWPs, SVPs, SSs AND FLOOR GULLEYS ARE TO HAVE RODDABLE ACCESS POINTS ABOVE GROUND FLOOR TO BUILDING SERVICES DETAILS.

6. THE CONTRACTOR IS TO UNDERTAKE A FULL CCTV SURVEY AT COMPLETION OF THE PROJECT TO DEMONSTRATE THAT THE DRAINAGE IS CLEAN. ANY DEFECTS ARE TO BE CORRECTED BY THE CONTRACTOR PRIOR TO HANDOVER OF THE SYSTEM TO THE CLIENT.

7. THE CONTRACTOR IS TO KEEP A RECORD SET OF THE AS INSTALLED DRAINAGE ON SITE AND PROVIDE THIS TO RENAISSANCE UPON COMPLETION OF THE WORKS TO ALLOW FOR PRODUCTION OF A FINAL CONSTRUCTION ISSUE DRAWING.

8. DURING THE WORKS ALTERATIONS TO THE DRAINAGE LAYOUT ARE TO BE MARKED UP BY THE CONTRACTOR AND AGREED WITH RENAISSANCE IN WRITING.

9. THE CHAMBER SIZE OF PCC MANHOLES WITH MORE THAN ONE CONNECTION IN THEM MAY NEED TO BE INCREASED AN INCREMENT TO ACCOMMODATE THE CONNECTIONS AND BENDS.

10. MANHOLE COVERS ARE TO BE IN ACCORDANCE WITH BS EN 124 FOR THE RATING INDICATED IN THE MANHOLE SCHEDULE. THE CONTRACTOR IS TO SUBMIT DETAILS FOR THE PROPOSED MANHOLE COVERS TO THE ENGINEER AND LANDSCAPE ARCHITECT FOR REVIEW PRIOR TO INSTALLATION.

11. ALL INTERNAL COVERS TO BE RECESSED AND TRIPLE SEALED

12. COVER SLABS MUST CARRY THE BSI KITEMARK, WHERE THE CLEAR OPENING OF THE KITEMARKED PRODUCT IS DIFFERENT TO THAT OF THE COVER AND FRAME, A LOAD BEARING REDUCING SLAB SHOULD BE FITTED ABOVE THE COVER SLAB TO BRING THE SIZE DOWN TO REQUIRED SIZE.

13. ALL SEWERS TO BE BSI KITEMARK (CERTIFIED TO WIS 4-35-01)

14. ALL CONCRETE PIPES TO BE TO ES EN 1916 CLASS 120 (54kNm).

15. BEDDING AND BACKFILL MATERIAL TO CONFORM TO THE REQUIREMENTS OF THE WATER INDUSTRY SPECIFICATION 4-08092 (TABLE A2).

16. IF PLASTIC PIPES ARE TO BE USED THEN THE FOLLOWING MUST APPLY:
A. BSI KITEMARK (CERTIFIED TO BS EN 13476-1 AND WIS 4-35-01)
B. PIPES LESS THAN OR EQUAL TO 500mm DIA
C. BEDDING AND BACKFILL MATERIAL TO CONFORM TO THE REQUIREMENTS OF THE WATER INDUSTRY SPECIFICATION 4-08092 (TABLE A2)

17. THE MINIMUM CRUSHING STRENGTH FOR CLAY PIPES SHOULD BE AS FOLLOWS: 100mm DIA. 40kNm, 150mm DIA. 40kNm, 225mm DIA. 45kNm AND 300mm DIA. 72kNm.

18. THE MINIMUM CRUSHING STRENGTH FOR CONCRETE PIPES SHOULD BE - (CLASS 120 TO EN 1916/BS5911-1 2002). PLASTIC PIPES SHOULD CONFORM TO WIS 4-35-01 AND BS EN13476.

19. SULPHATE RESISTANT CEMENT (C20-DC3) AND PRECAST CONCRETE PRODUCTS MUST BE USED OR A LABORATORY REPORT PROVIDING THAT SUCH PRECAUTIONS ARE NOT NECESSARY.

20. SEWER PIPES TO BE LAID IN MAXIMUM 3 METER LENGTHS UNLESS THERE IS A SPECIFIC OPERATIONAL NEED TO LAY LONGER PIPES, AND THIS IS AGREED BY THE CONTRACTOR WITH THE PIPE MANUFACTURER.

21. ALL FOUL AND SURFACE WATER BELOW GROUND SYSTEMS ARE TO BE VENTED WITHIN THE ABOVE GROUND DRAINAGE TO THE BUILDING SERVICES ENGINEERS DETAILS.

EXTERNAL WORKS SETTING OUT

FOR PROPOSED SITE LEVELS AND CHANNEL / GULLY LOCATIONS, REFER TO LANDSCAPE ARCHITECT'S DRAWINGS.

SVp & RWP SETTING OUT

SVp AND RWP LOCATIONS ARE INDICATIVE. FOR DETAILED SETTING OUT REFER TO ARCHITECTS/ BUILDING SERVICE ENGINEERS DRAWINGS.

ALL CONNECTIONS BETWEEN ABOVE GROUND DRAINAGE AND BELOW GROUND DRAINAGE TO BE SIZED TO SUIT ABOVE GROUND DRAINAGE.

METROLINK LINE

ALL DRAINAGE AND SURFACE FINISHES ADJACENT TO THE METROLINK LINE TO THE EAST OF THE SITE ARE TO FALL AWAY FROM THE EMBANKMENT TO PREVENT ANY IMPACTS ON ITS STABILITY.

PITCH DRAINAGE

THE PITCH AREAS HAVE BEEN INCLUDED WITHIN THE PROPOSED DRAINAGE DESIGN ALLOWING FOR THE FLOW RATES ADVISED BY THE SUDS SPECIALIST. THERE IS POTENTIAL FOR THE PITCHES TO BE DRAINED VIA AN INFILTRATION ONLY SYSTEM. THIS IS TO BE CONFIRMED ONCE ADDITIONAL INFILTRATION TESTING CAN BE CARRIED OUT UNDER THE FOOTPRINT OF THE EXISTING LEISURE CENTRE BUILDING.

P04	MANHOLE INVERT LEVELS UPDATED	13.12.23	SA	PW
P03	STAGE 4 ISSUE	08.12.23	SA	PW
P02	COMMENTS INCORPORATED	08.09.23	HW	PW
P01	STAGE 3 ISSUE	11.08.23	SA	PW
Rev:	Description:	Date:	By:	Chkd:

renaissance

Project:

STAR RADCLIFFE ACADEMY, BURY

Drawing Title:

STAGE 4

Status:

Size:	Date:	Drawn by:	Designed by:	Checked by:
A1	DEC 23	SA	SA	PW

Scale:

1:500

Project No:

2303-02

Project - Originator - Volume - Level - Type - Role - Number:

Revision:

FS1013-REN-EX-00-D-C-5210

P04

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Renaissance Associates Ltd
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Project:

STAR RADCLIFFE ACADEMY, BURY

Drawing Title:

BELOW GROUND DRAINAGE PLAN

Status:

STAGE 4

Size:	Date:	Drawn by:	Designed by:	Checked by:
A1	DEC 23	SA	SA	PW

Scale:

1:500

Project No:

2303-02

Project - Originator - Volume - Level - Type - Role - Number:

Revision:

FS1013-REN-EX-00-D-C-5210

P04

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APPENDIX J
Proposed Surface Water Micro Drainage Calculations

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Carvers Warehouse 77 Dale Street Manchester M1 2HG		
Date 13/12/2023 16:23 File 231213MD model 2.MDX	Designed by SelvanaAhmad Checked by	
Innovyze	Network 2020.1.3	

STORM SEWER DESIGN by the Modified Rational Method

Design Criteria for Storm

Pipe Sizes STANDARD Manhole Sizes STANDARD

FSR Rainfall Model - England and Wales

Return Period (years)	100	PIMP (%)	100
M5-60 (mm)	18.000	Add Flow / Climate Change (%)	0
Ratio R	0.360	Minimum Backdrop Height (m)	0.200
Maximum Rainfall (mm/hr)	50	Maximum Backdrop Height (m)	1.500
Maximum Time of Concentration (mins)	30	Min Design Depth for Optimisation (m)	1.200
Foul Sewage (l/s/ha)	0.000	Min Vel for Auto Design only (m/s)	1.00
Volumetric Runoff Coeff.	0.750	Min Slope for Optimisation (1:X)	500

Designed with Level Soffits

Time Area Diagram for Storm

Time (mins)	Area (ha)	Time (mins)	Area (ha)
0-4	0.238	4-8	0.117

Total Area Contributing (ha) = 0.355

Total Pipe Volume (m³) = 46.695

Network Design Table for Storm

- Indicates pipe length does not match coordinates

PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	Base Flow (l/s)	k (mm)	HYD SECT	DIA (mm)	Section Type	Auto Design
S1.000	39.722	0.209	190.1	0.000	5.00	0.0	0.600	o	375	Pipe/Conduit	
S1.001	26.346	0.132	199.6	0.000	0.00	0.0	0.600	o	375	Pipe/Conduit	
S1.002	25.923	0.130	199.4	0.000	0.00	0.0	0.600	o	375	Pipe/Conduit	
S2.000	35.107	0.176	200.0	0.000	5.00	0.0	0.600	o	150	Pipe/Conduit	

Network Results Table

PN	Rain (mm/hr)	T.C. (mins)	US/IL (m)	Σ I.Area (ha)	Σ Base Flow (l/s)	Foul (l/s)	Add Flow (l/s)	Vel (m/s)	Cap (l/s)	Flow (l/s)
S1.000	50.00	5.51	73.189	0.000	0.0	0.0	0.0	1.31	144.8	0.0
S1.001	50.00	5.85	72.980	0.000	0.0	0.0	0.0	1.28	141.2	0.0
S1.002	50.00	6.19	72.848	0.000	0.0	0.0	0.0	1.28	141.3	0.0
S2.000	50.00	5.83	73.224	0.000	0.0	0.0	0.0	0.71	12.5	0.0

Renaissance Assoc.

Carvers Warehouse
77 Dale Street
Manchester M1 2HG

Date 13/12/2023 16:23
File 231213MD model 2.MDX

Innovyze

Designed by SelvanaAhmad
Checked by

Network 2020.1.3

Page 3



Network Design Table for Storm

PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	Base Flow (l/s)	k (mm)	HYD SECT	DIA (mm)	Section Type	Auto Design
S4.006	29.780	1.424	20.9	0.055	0.00	0.0	0.600	o	300	Pipe/Conduit	
S1.008	44.158	0.442	100.0	0.000	0.00	0.0	0.600	o	375	Pipe/Conduit	
S1.009	16.130	0.319	50.5	0.000	0.00	0.0	0.600	o	375	Pipe/Conduit	
S7.000	15.803	0.105	150.0	0.000	5.00	1.6	0.600	o	150	Pipe/Conduit	
S1.010	37.351#	0.187	200.0	0.000	0.00	0.0	0.600	o	375	Pipe/Conduit	
S1.011	16.322	0.375	43.5	0.000	0.00	0.0	0.600	o	375	Pipe/Conduit	
S8.000	20.336	0.203	100.0	0.028	5.00	0.0	0.600	o	225	Pipe/Conduit	
S8.001	17.578	0.531	33.1	0.063	0.00	0.0	0.600	o	225	Pipe/Conduit	
S9.000	9.636	0.859	11.2	0.012	5.00	0.0	0.600	o	150	Pipe/Conduit	
S1.012	10.044	2.760	3.6	0.000	0.00	0.0	0.600	o	375	Pipe/Conduit	

Network Results Table

PN	Rain (mm/hr)	T.C. (mins)	US/IL (m)	E I.Area (ha)	E Base Flow (l/s)	Foul (l/s)	Add Flow (l/s)	Vel (m/s)	Cap (l/s)	Flow (l/s)
S4.006	50.00	6.70	71.062	0.252	3.9	0.0	0.0	3.45	244.1	38.0
S1.008	50.00	7.44	69.563	0.252	12.8	0.0	0.0	1.81	200.1	46.9
S1.009	50.00	7.55	69.122	0.252	12.8	0.0	0.0	2.55	282.1	46.9
S7.000	50.00	5.32	69.150	0.000	1.6	0.0	0.0	0.82	14.5	1.6
S1.010	50.00	8.03	68.803	0.252	14.4	0.0	0.0	1.28	141.1	48.5
S1.011	50.00	8.13	68.616	0.252	14.4	0.0	0.0	2.75	304.2	48.5
S8.000	50.00	5.26	69.050	0.028	0.0	0.0	0.0	1.31	52.0	3.8
S8.001	50.00	5.39	68.847	0.091	0.0	0.0	0.0	2.28	90.7	12.3
S9.000	50.00	5.05	69.250	0.012	0.0	0.0	0.0	3.03	53.5	1.6
S1.012	50.00	8.15	68.166	0.355	14.4	0.0	0.0	9.55	1055.1	62.5

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Date 13/12/2023 16:23 File 231213MD model 2.MDX	Designed by SelvanaAhmad Checked by	
Innovyze	Network 2020.1.3	

Area Summary for Storm

Pipe Number	PIMP Type	PIMP Name	PIMP (%)	Gross Area (ha)	Imp. Area (ha)	Pipe Total (ha)
1.000	-	-	100	0.000	0.000	0.000
1.001	-	-	100	0.000	0.000	0.000
1.002	-	-	100	0.000	0.000	0.000
2.000	-	-	100	0.000	0.000	0.000
2.001	-	-	100	0.000	0.000	0.000
1.003	-	-	100	0.000	0.000	0.000
1.004	-	-	100	0.000	0.000	0.000
3.000	-	-	100	0.000	0.000	0.000
1.005	-	-	100	0.000	0.000	0.000
1.006	-	-	100	0.000	0.000	0.000
1.007	-	-	100	0.000	0.000	0.000
4.000	-	-	100	0.016	0.016	0.016
4.001	-	-	100	0.017	0.017	0.017
4.002	-	-	100	0.000	0.000	0.000
4.003	-	-	100	0.000	0.000	0.000
4.004	-	-	100	0.164	0.164	0.164
5.000	-	-	100	0.000	0.000	0.000
4.005	-	-	100	0.000	0.000	0.000
6.000	-	-	100	0.000	0.000	0.000
4.006	-	-	100	0.055	0.055	0.055
1.008	-	-	100	0.000	0.000	0.000
1.009	-	-	100	0.000	0.000	0.000
7.000	-	-	100	0.000	0.000	0.000
1.010	-	-	100	0.000	0.000	0.000
1.011	-	-	100	0.000	0.000	0.000
8.000	-	-	100	0.028	0.028	0.028
8.001	-	-	100	0.063	0.063	0.063
9.000	-	-	100	0.012	0.012	0.012
1.012	-	-	100	0.000	0.000	0.000
				Total	Total	Total
				0.355	0.355	0.355

Free Flowing Outfall Details for Storm

Outfall Pipe Number	Outfall Name	C. Level (m)	I. Level (m)	Min I. Level (m)	D,L (mm)	W (mm)
S1.012	S	70.160	65.406	0.000	0	0

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Date 13/12/2023 16:23 File 231213MD model 2.MDX	Designed by SelvanaAhmad Checked by	
Innovyze Network 2020.1.3		

Simulation Criteria for Storm

Volumetric Runoff Coeff	1.000	Additional Flow - % of Total Flow	0.000
Areal Reduction Factor	1.000	MADD Factor * 10m ³ /ha Storage	2.000
Hot Start (mins)	0	Inlet Coeffiecient	0.800
Hot Start Level (mm)	0	Flow per Person per Day (l/per/day)	0.000
Manhole Headloss Coeff (Global)	0.500	Run Time (mins)	60
Foul Sewage per hectare (l/s)	0.000	Output Interval (mins)	1

Number of Input Hydrographs	0	Number of Storage Structures	1
Number of Online Controls	1	Number of Time/Area Diagrams	0
Number of Offline Controls	0	Number of Real Time Controls	0

Synthetic Rainfall Details

Rainfall Model	FSR	Profile Type	Summer
Return Period (years)	100	Cv (Summer)	1.000
Region	England and Wales	Cv (Winter)	0.840
M5-60 (mm)	18.000	Storm Duration (mins)	30
Ratio R	0.360		

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Innovyze	Network 2020.1.3																																																																									
Online Controls for Storm Hydro-Brake® Optimum Manhole: S1.12, DS/PN: S1.012, Volume (m³): 6.0 Unit Reference MD-SCL-0277-5970-2600-5970 Design Head (m)2.600 Design Flow (l/s)59.7 Flush-Flo™Calculated ObjectiveMinimise blockage risk ApplicationSurface Sump AvailableYes Diameter (mm)277 Invert Level (m)68.166 Minimum Outlet Pipe Diameter (mm)300 Suggested Manhole Diameter (mm)2100 Control Points Head (m)Flow (l/s) Design Point (Calculated)2.60059.7 Flush-Flo™0.56159.6 Kick-Flo®1.40644.4 Mean Flow over Head Range-50.3 The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated <table><tr><td>Depth (m)</td><td>Flow (l/s)</td><td>Depth (m)</td><td>Flow (l/s)</td><td>Depth (m)</td><td>Flow (l/s)</td><td>Depth (m)</td><td>Flow (l/s)</td></tr><tr><td>0.100</td><td>9.6</td><td>1.200</td><td>52.0</td><td>3.000</td><td>63.9</td><td>7.000</td><td>96.4</td></tr><tr><td>0.200</td><td>31.5</td><td>1.400</td><td>44.8</td><td>3.500</td><td>68.9</td><td>7.500</td><td>99.7</td></tr><tr><td>0.300</td><td>51.0</td><td>1.600</td><td>47.3</td><td>4.000</td><td>73.5</td><td>8.000</td><td>102.8</td></tr><tr><td>0.400</td><td>58.3</td><td>1.800</td><td>50.0</td><td>4.500</td><td>77.8</td><td>8.500</td><td>105.9</td></tr><tr><td>0.500</td><td>59.5</td><td>2.000</td><td>52.6</td><td>5.000</td><td>81.9</td><td>9.000</td><td>108.9</td></tr><tr><td>0.600</td><td>59.6</td><td>2.200</td><td>55.1</td><td>5.500</td><td>85.7</td><td>9.500</td><td>111.8</td></tr><tr><td>0.800</td><td>58.1</td><td>2.400</td><td>57.4</td><td>6.000</td><td>89.4</td><td></td><td></td></tr><tr><td>1.000</td><td>55.7</td><td>2.600</td><td>59.7</td><td>6.500</td><td>93.0</td><td></td><td></td></tr></table> ©1982-2020 Innovyze			Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	0.100	9.6	1.200	52.0	3.000	63.9	7.000	96.4	0.200	31.5	1.400	44.8	3.500	68.9	7.500	99.7	0.300	51.0	1.600	47.3	4.000	73.5	8.000	102.8	0.400	58.3	1.800	50.0	4.500	77.8	8.500	105.9	0.500	59.5	2.000	52.6	5.000	81.9	9.000	108.9	0.600	59.6	2.200	55.1	5.500	85.7	9.500	111.8	0.800	58.1	2.400	57.4	6.000	89.4			1.000	55.7	2.600	59.7	6.500	93.0		
Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)																																																																			
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1.000	55.7	2.600	59.7	6.500	93.0																																																																					

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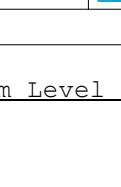
Storage Structures for Storm

Tank or Pond Manhole: S1.10, DS/PN: S1.010

Invert Level (m) 68.820

Depth (m)	Area (m ²)	Depth (m)	Area (m ²)
0.000	180.0	1.200	552.0

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1 year Return Period Summary of Critical Results by Maximum Level (Rank 1)
for Storm

Simulation Criteria

Areal Reduction Factor	1.000	Additional Flow - % of Total Flow	0.000
Hot Start (mins)	0	MADD Factor * 10m³/ha Storage	2.000
Hot Start Level (mm)	0	Inlet Coeffiecient	0.800
Manhole Headloss Coeff (Global)	0.500	Flow per Person per Day (l/per/day)	0.000
Foul Sewage per hectare (l/s)	0.000		

Number of Input Hydrographs	0	Number of Storage Structures	1
Number of Online Controls	1	Number of Time/Area Diagrams	0
Number of Offline Controls	0	Number of Real Time Controls	0

Synthetic Rainfall Details

Rainfall Model	FSR	Ratio R	0.360
Region England and Wales Cv (Summer)	1.000		
M5-60 (mm)	18.000	Cv (Winter)	1.000

Margin for Flood Risk Warning (mm)	300.0
Analysis Timestep	2.5 Second Increment (Extended)
DTS Status	OFF
DVD Status	ON
Inertia Status	ON

Profile(s)	Summer and Winter
Duration(s) (mins)	15, 30, 60, 120, 180, 240, 360
Return Period(s) (years)	1, 2, 30, 100
Climate Change (%)	0, 0, 0, 45

PN	US/MH Name	Storm	Return Period	Climate Change	First (X) Surge	First (Y) Flood	First (Z) Overflow	Overflow Act.
S1.000	S1.00	15 Summer	1	+0%				
S1.001	S1.01	15 Summer	1	+0%				
S1.002	S1.02	15 Summer	1	+0%				
S2.000	S2.00	15 Summer	1	+0%				
S2.001	S2.01	15 Summer	1	+0%				
S1.003	S1.03	15 Summer	1	+0%				
S1.004	S1.04	15 Summer	1	+0%				
S3.000	S3.00	15 Summer	1	+0%				
S1.005	S1.05	15 Summer	1	+0%				
S1.006	S1.06	15 Summer	1	+0%				
S1.007	S1.07	15 Summer	1	+0%				
S4.000	S4.00	15 Summer	1	+0%				
S4.001	S4.01	15 Summer	1	+0%				
S4.002	S4.02	15 Summer	1	+0%				
S4.003	S4.03	15 Summer	1	+0%	100/15 Summer			
S4.004	S4.04	15 Summer	1	+0%	100/15 Summer			
S5.000	SS3G PITCH	15 Summer	1	+0%				
S4.005	S4.05	15 Summer	1	+0%				
S6.000	S3G MUGA	15 Summer	1	+0%				

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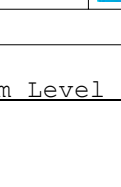
1 year Return Period Summary of Critical Results by Maximum Level (Rank 1)

for Storm

PN	US/MH Name	Water Level (m)	Surcharged Depth (m)	Flooded Volume (m³)	Flow / Overflow Cap. (l/s)	Half Drain Time (mins)	Pipe Flow (l/s)	Status
S1.000	S1.00	73.189	-0.375	0.000	0.00		0.0	OK
S1.001	S1.01	72.980	-0.375	0.000	0.00		0.0	OK
S1.002	S1.02	72.848	-0.375	0.000	0.00		0.0	OK
S2.000	S2.00	73.224	-0.150	0.000	0.00		0.0	OK
S2.001	S2.01	73.048	-0.150	0.000	0.00		0.0	OK
S1.003	S1.03	72.718	-0.375	0.000	0.00		0.0	OK
S1.004	S1.04	72.676	-0.333	0.000	0.03		8.9	OK
S3.000	S3.00	72.450	-0.150	0.000	0.00		0.0	OK
S1.005	S1.05	71.788	-0.333	0.000	0.03		9.0	OK
S1.006	S1.06	70.044	-0.312	0.000	0.06		8.9	OK
S1.007	S1.07	69.900	-0.321	0.000	0.05		9.0	OK
S4.000	S4.00	73.368	-0.192	0.000	0.05		2.4	OK
S4.001	S4.01	73.113	-0.177	0.000	0.10		4.6	OK
S4.002	S4.02	72.938	-0.179	0.000	0.09		4.6	OK
S4.003	S4.03	72.552	-0.178	0.000	0.10		4.6	OK
S4.004	S4.04	72.403	-0.182	0.000	0.32		25.1	OK
S5.000	SS3G PITCH	72.830	-0.120	0.000	0.09		3.0	OK
S4.005	S4.05	72.264	-0.221	0.000	0.15		28.3	OK
S6.000	S3G MUGA	71.323	-0.202	0.000	0.02		0.9	OK

PN	US/MH Name	Level Exceeded
S1.000	S1.00	
S1.001	S1.01	
S1.002	S1.02	
S2.000	S2.00	
S2.001	S2.01	
S1.003	S1.03	
S1.004	S1.04	
S3.000	S3.00	
S1.005	S1.05	
S1.006	S1.06	
S1.007	S1.07	
S4.000	S4.00	
S4.001	S4.01	
S4.002	S4.02	
S4.003	S4.03	
S4.004	S4.04	
S5.000	SS3G PITCH	
S4.005	S4.05	
S6.000	S3G MUGA	

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2 year Return Period Summary of Critical Results by Maximum Level (Rank 1)
for Storm

Simulation Criteria

Areal Reduction Factor	1.000	Additional Flow - % of Total Flow	0.000
Hot Start (mins)	0	MADD Factor * 10m³/ha Storage	2.000
Hot Start Level (mm)	0	Inlet Coeffiecient	0.800
Manhole Headloss Coeff (Global)	0.500	Flow per Person per Day (l/per/day)	0.000
Foul Sewage per hectare (l/s)	0.000		

Number of Input Hydrographs	0	Number of Storage Structures	1
Number of Online Controls	1	Number of Time/Area Diagrams	0
Number of Offline Controls	0	Number of Real Time Controls	0

Synthetic Rainfall Details

Rainfall Model	FSR	Ratio R	0.360
Region England and Wales Cv (Summer)	1.000		
M5-60 (mm)	18.000	Cv (Winter)	1.000

Margin for Flood Risk Warning (mm)	300.0
Analysis Timestep	2.5 Second Increment (Extended)
DTS Status	OFF
DVD Status	ON
Inertia Status	ON

Profile(s)	Summer and Winter
Duration(s) (mins)	15, 30, 60, 120, 180, 240, 360
Return Period(s) (years)	1, 2, 30, 100
Climate Change (%)	0, 0, 0, 45

PN	US/MH Name	Storm	Return Period	Climate Change	First (X) Surge	First (Y) Flood	First (Z) Overflow	Overflow Act.
S1.000	S1.00	15 Summer	2	+0%				
S1.001	S1.01	15 Summer	2	+0%				
S1.002	S1.02	15 Summer	2	+0%				
S2.000	S2.00	15 Summer	2	+0%				
S2.001	S2.01	15 Summer	2	+0%				
S1.003	S1.03	15 Summer	2	+0%				
S1.004	S1.04	15 Summer	2	+0%				
S3.000	S3.00	15 Summer	2	+0%				
S1.005	S1.05	15 Summer	2	+0%				
S1.006	S1.06	15 Summer	2	+0%				
S1.007	S1.07	15 Summer	2	+0%				
S4.000	S4.00	15 Summer	2	+0%				
S4.001	S4.01	15 Summer	2	+0%				
S4.002	S4.02	15 Summer	2	+0%				
S4.003	S4.03	15 Summer	2	+0%	100/15 Summer			
S4.004	S4.04	15 Summer	2	+0%	100/15 Summer			
S5.000	SS3G PITCH	15 Summer	2	+0%				
S4.005	S4.05	15 Summer	2	+0%				
S6.000	S3G MUGA	15 Summer	2	+0%				

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30 year Return Period Summary of Critical Results by Maximum Level (Rank 1)

for Storm

Simulation Criteria

Areal Reduction Factor 1.000

Hot Start (mins) 0

Hot Start Level (mm) 0

Manhole Headloss Coeff (Global) 0.500

Foul Sewage per hectare (l/s) 0.000

Additional Flow - % of Total Flow 0.000

MADD Factor * 10m³/ha Storage 2.000

Inlet Coeffiecient 0.800

Flow per Person per Day (l/per/day) 0.000

Number of Input Hydrographs 0

Number of Online Controls 1

Number of Offline Controls 0

Number of Storage Structures 1

Number of Time/Area Diagrams 0

Number of Real Time Controls 0

Synthetic Rainfall Details

Rainfall Model FSR

Region England and Wales Cv (Summer) 1.000

M5-60 (mm) 18.000

Ratio R 0.360

Cv (Winter) 1.000

Margin for Flood Risk Warning (mm) 300.0

Analysis Timestep 2.5 Second Increment (Extended)

DTS Status OFF

DVD Status ON

Inertia Status ON

Profile(s) Summer and Winter

Duration(s) (mins) 15, 30, 60, 120, 180, 240, 360

Return Period(s) (years) 1, 2, 30, 100

Climate Change (%) 0, 0, 0, 45

PN	US/MH Name	Storm	Return Period	Climate Change	First (X) Surge	First (Y) Flood	First (Z) Overflow	Overflow Act.
S1.000	S1.00	15 Summer	30	+0%				
S1.001	S1.01	15 Summer	30	+0%				
S1.002	S1.02	15 Summer	30	+0%				
S2.000	S2.00	15 Summer	30	+0%				
S2.001	S2.01	15 Summer	30	+0%				
S1.003	S1.03	15 Summer	30	+0%				
S1.004	S1.04	15 Summer	30	+0%				
S3.000	S3.00	15 Summer	30	+0%				
S1.005	S1.05	15 Summer	30	+0%				
S1.006	S1.06	15 Summer	30	+0%				
S1.007	S1.07	15 Summer	30	+0%				
S4.000	S4.00	15 Summer	30	+0%				
S4.001	S4.01	15 Summer	30	+0%				
S4.002	S4.02	15 Summer	30	+0%				
S4.003	S4.03	15 Summer	30	+0%	100/15 Summer			
S4.004	S4.04	15 Summer	30	+0%	100/15 Summer			
S5.000	SS3G PITCH	15 Summer	30	+0%				
S4.005	S4.05	15 Summer	30	+0%				
S6.000	S3G MUGA	15 Summer	30	+0%				

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30 year Return Period Summary of Critical Results by Maximum Level (Rank 1)
for Storm

PN	US/MH Name	Water Level (m)	Surcharged Depth (m)	Flooded Volume (m³)	Flow / Cap. (l/s)	Half Drain Time (mins)	Pipe Flow (l/s)	Status
S1.000	S1.00	73.189	-0.375	0.000	0.00		0.0	OK
S1.001	S1.01	72.980	-0.375	0.000	0.00		0.0	OK
S1.002	S1.02	72.848	-0.375	0.000	0.00		0.0	OK
S2.000	S2.00	73.224	-0.150	0.000	0.00		0.0	OK
S2.001	S2.01	73.048	-0.150	0.000	0.00		0.0	OK
S1.003	S1.03	72.718	-0.375	0.000	0.00		0.0	OK
S1.004	S1.04	72.676	-0.333	0.000	0.03		8.9	OK
S3.000	S3.00	72.450	-0.150	0.000	0.00		0.0	OK
S1.005	S1.05	71.788	-0.333	0.000	0.03		9.0	OK
S1.006	S1.06	70.044	-0.312	0.000	0.06		8.9	OK
S1.007	S1.07	69.900	-0.321	0.000	0.05		9.0	OK
S4.000	S4.00	73.388	-0.172	0.000	0.12		5.9	OK
S4.001	S4.01	73.147	-0.144	0.000	0.28		13.0	OK
S4.002	S4.02	72.970	-0.146	0.000	0.26		12.6	OK
S4.003	S4.03	72.589	-0.141	0.000	0.28		12.8	OK
S4.004	S4.04	72.549	-0.037	0.000	1.00		79.4	OK
S5.000	SS3G PITCH	72.830	-0.120	0.000	0.09		3.0	OK
S4.005	S4.05	72.326	-0.159	0.000	0.45		82.2	OK
S6.000	S3G MUGA	71.323	-0.202	0.000	0.02		0.9	OK

PN	US/MH Name	Level Exceeded
S1.000	S1.00	
S1.001	S1.01	
S1.002	S1.02	
S2.000	S2.00	
S2.001	S2.01	
S1.003	S1.03	
S1.004	S1.04	
S3.000	S3.00	
S1.005	S1.05	
S1.006	S1.06	
S1.007	S1.07	
S4.000	S4.00	
S4.001	S4.01	
S4.002	S4.02	
S4.003	S4.03	
S4.004	S4.04	
S5.000	SS3G PITCH	
S4.005	S4.05	
S6.000	S3G MUGA	

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<u>30 year Return Period Summary of Critical Results by Maximum Level (Rank 1)</u> <u>for Storm</u>									
PN	US/MH Name	Storm	Return Period	Climate Change	First (X) Surcharge	First (Y) Flood	First (Z) Overflow	Overflow Act.	
S4.006	S4.06	15 Summer	30	+0%					
S1.008	S1.08	15 Summer	30	+0%	100/15 Summer				
S1.009	S1.09	15 Summer	30	+0%					
S7.000	S2G PITCH	15 Summer	30	+0%					
S1.010	S1.10	30 Summer	30	+0%	100/30 Summer				
S1.011	S1.11	30 Summer	30	+0%	100/15 Summer				
S8.000	S5.00	15 Summer	30	+0%	100/15 Summer				
S8.001	S5.01	15 Summer	30	+0%	100/15 Summer				
S9.000	S6.00	15 Summer	30	+0%					
S1.012	S1.12	60 Summer	30	+0%	30/15 Summer				
PN	US/MH Name	Water Level (m)	Surcharged Depth (m)	Flooded Volume (m³)	Flow / Cap.	Overflow (l/s)	Half Drain Time (mins)	Pipe Flow (l/s)	Status
S4.006	S4.06	71.209	-0.154	0.000	0.48			105.8	OK
S1.008	S1.08	69.779	-0.159	0.000	0.62			113.5	OK
S1.009	S1.09	69.312	-0.184	0.000	0.51			113.5	OK
S7.000	S2G PITCH	69.184	-0.116	0.000	0.12			1.6	OK
S1.010	S1.10	68.994	-0.184	0.000	0.47			59.3	OK
S1.011	S1.11	68.960	-0.030	0.000	0.23			54.7	OK
S8.000	S5.00	69.122	-0.153	0.000	0.22			10.4	OK
S8.001	S5.01	68.953	-0.118	0.000	0.45			36.8	OK
S9.000	S6.00	69.281	-0.119	0.000	0.10			4.5	OK
S1.012	S1.12	68.873	0.332	0.000	0.09			59.6	SURCHARGED
PN	US/MH Name	Level Exceeded							
S4.006	S4.06								
S1.008	S1.08								
S1.009	S1.09								
S7.000	S2G PITCH								
S1.010	S1.10								
S1.011	S1.11								
S8.000	S5.00								
S8.001	S5.01								
S9.000	S6.00								
S1.012	S1.12								
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