

Drainage Strategy Statement

Former Ambulance Station

Cemetery Road, Ramsbottom, Bury, BL0 9PU

Document Control

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Client: Home Park Avenue Ltd

Revision: Rev B

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Local Planning Authority: Bury Council

HOME GROUP
CONSULTING ENGINEERING



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1. Introduction

This Drainage Strategy Statement is submitted in support of a full planning application for the redevelopment of the Former Ambulance Station, Cemetery Road, Ramsbottom, to provide 3 no. detached dwellings. The statement follows the national drainage hierarchy and provides justification for the proposed connection arrangement based on a reduction in impermeable area compared with the former ambulance station use.

2. Site and Proposed Development

The proposals comprise three detached dwellings with associated access, parking and landscaping. The development replaces an existing previously developed use and reduces the extent of impermeable hardstanding across the site, thereby reducing surface water runoff volumes in comparison with the existing arrangement.

3. Existing Drainage and United Utilities Records

United Utilities (UU) asset records have been obtained for the site locality. The UU Sewer Records plan confirms public drainage infrastructure within the surrounding highway network. The development intends to make a connection on site to the existing manholes serving the former ambulance station (existing points of discharge), subject to verification by survey/CCTV and formal UU connection consent.

Appendix A provides an extract of the UU Sewer Records plan for reference. Asset locations shown are approximate and must be confirmed on site prior to construction.

4. Drainage Hierarchy Assessment (NPPF/NPPG)

In accordance with the National Planning Policy Framework and National Planning Practice Guidance, surface water disposal has been considered in the following order of priority:

- 1) Infiltration to ground (e.g., soakaways/infiltration basins)
- 2) Discharge to a surface water body
- 3) Discharge to a surface water sewer, highway drain or another drainage system
- 4) Discharge to a combined sewer

Infiltration (soakaways): Ground conditions are understood to comprise sandy / clay at circa 1m depth. Clay soils typically have low permeability and are not suitable for infiltration drainage. On this basis, and without prejudice to any further BRE365 testing, infiltration/soakaways are not considered feasible or reliable for this site.

we have identified low-permeability ground conditions indicate that conventional soakaways are not considered suitable as the primary means of surface water disposal. Notwithstanding this, permeable driveway construction is proposed as a source-control measure. The permeable pavement will provide temporary storage within the open-graded sub-base, reduce the rate and volume of immediate surface runoff, and allow incidental infiltration where local ground conditions permit. The design does not rely on infiltration to ground for full disposal of surface water; positive drainage or controlled overflow is provided where required.

Surface water body: No suitable surface water body has been identified within or adjacent to the site boundary for direct discharge.

Surface water sewer/highway drain: A separate surface water sewer connection has not been confirmed as available at this stage and would be subject to further statutory undertaker confirmation.

Combined sewer: Accordingly, the proposed strategy is to continue to use the established drainage route by connecting (via the existing on-site manholes) into the existing network, subject to UU approval. This is considered proportionate and acceptable as the proposal reduces impermeable area and does not increase surface water burden compared to the former use. this in conjunction with the temporary storage is deemed sufficient.

5. Proposed Drainage Strategy (Foul and Surface Water)

Foul water: Each dwelling will discharge foul flows via private drainage to on-site inspection chambers/manholes and connect to existing manholes serving the former ambulance station. Connection to the public sewer network will be progressed via the appropriate statutory process (Section 106 connection consent), with detailed design confirmed at Building Regulations stage.

Surface water: Roof and paved area runoff will discharge to on-site pipework and manholes and connect to the existing manholes/outfall used historically by the ambulance station. Where practicable, permeable surfacing will be utilised to provide source control and reduce runoff. As the proposal reduces impermeable area compared to the baseline, dedicated on-site attenuation is not required for this planning-stage strategy (see Section 6).

6. Surface Water Runoff Comparison and Hardstanding Reduction (calculations)

The following comparative calculation uses the previously agreed site figures to demonstrate reduced impermeable area and runoff volume for a 50mm event (0.05m rainfall depth).

Scenario	Impermeable area (m ²)	Rainfall depth (m)	Runoff volume (m ³)
Existing / baseline hardstanding	520	0.05	26
Proposed hardstanding	220	0.05	21
Reduction	19		5

This demonstrates an impermeable area reduction of **300m²** and a corresponding runoff volume reduction of **5.00 m³** for a 50mm event, equating to approximately **57%** reduction relative to the baseline. On this basis, the development does not increase discharge potential compared to the former use and on-site attenuation is not required as part of this planning-stage strategy.

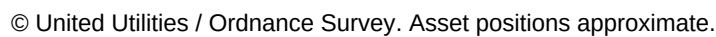
7. Construction, Adoption and Approvals

existing drainage arrangement by CCTV survey confirming manhole locations, inverts and condition. A formal application for connection will be made to United Utilities under the relevant statutory process (Section 106 and/or Section 104 if any adoption is proposed). All works will maintain required easements and avoid building over/close to public sewers without written approval.

8. Maintenance and Management (summary)

Private drainage within the site will be maintained by the respective homeowner/management company. Routine inspections of gullies and manholes will be undertaken with clearance of debris and silt as required. Where permeable paving is provided, periodic sweeping/vacuuming will be undertaken to maintain permeability.

United Utilities Sewer Records plan extract (for guidance only; verify exact positions on site prior to works).



DRAINAGE STRATEGY

Former Ambulance Station

Cemetery Road, Ramsbottom, BL0 9PU

Proposed Residential Development

Prepared by
Andrew Gaskell BSc Civil Engineering
HOME Group NW Ltd
Revision P3 - Updated final drainage connection and chamber levels
September 2026

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1.0 Introduction

This Drainage Strategy has been prepared for the proposed residential redevelopment of the former ambulance station at Cemetery Road, Ramsbottom. It consolidates the foul and surface water drainage arrangements, pipe sizes, gradients, chamber levels, runoff reduction measures and permeable paving proposals for submission in support of discharge of the relevant planning drainage condition.

Revision P3 reflects the current proposed drainage layout in which SW1 and FW1 have been removed. SW2 and FW2 are now the final proposed inspection chambers and connect directly to the existing on-site combined inspection chamber approximately 4.600 m downstream. The existing combined chamber invert is 165.100 m AOD.

2.0 Site and Proposed Development

The site is the former ambulance station on Cemetery Road, Ramsbottom. The proposed development comprises three detached dwellings with associated private gardens, driveways, hardstanding and landscaping.

The existing site contains a substantial area of hardstanding associated with the former ambulance station. The proposed scheme reduces the overall hardstanding area from approximately 520 m² to approximately 220 m².

3.0 Existing Drainage and Ground Conditions

The current drainage plan identifies an existing on-site combined inspection chamber with cover level 165.780 m AOD and invert level 165.100 m AOD. This chamber is the immediate downstream receiving point for the new foul and surface water systems. The existing site drainage continues onward to the existing drainage network, including the downstream existing manhole shown on the site plan.

The Phase 1 environmental information describes superficial glacial till of generally low permeability. Conventional soakaways are therefore not relied upon as the primary means of surface water disposal. Permeable pavement is instead used as a source-control and attenuation measure, providing temporary storage within the open-graded pavement build-up and allowing incidental infiltration where local ground conditions permit.

The use of permeable paving does not imply that the site is suitable for conventional soakaway drainage. The permeable pavement is intended to reduce the rate and volume of immediate runoff and to provide temporary storage, with positive drainage/overflow provided where required.

The arboricultural method statement also requires permeable construction within relevant hardstanding areas, including open-graded aggregate and cellular confinement where works occur within tree root protection areas.

4.0 Drainage Design Principles

- Foul and surface water are conveyed in separate site drainage systems before discharging independently into the existing on-site combined inspection chamber.
- The fixed downstream control level for both proposed systems is the existing combined inspection chamber invert of 165.100 m AOD.
- SW2 and FW2 are the final proposed chambers; SW1 and FW1 have been removed from the design.
- Foul drainage is designed using 150 mm nominal pipework at a minimum gradient of 1:150 on the shared runs.
- Surface water drainage is designed using 100 mm nominal pipework at a minimum gradient of 1:100.
- Inspection chamber cover levels are coordinated with the latest proposed external levels.

- Permeable driveway construction is used to reduce direct runoff and provide source-control storage rather than as a conventional soakaway.
- Surface water is not permitted to discharge uncontrolled onto the public footway/highway.
- All drainage is to be installed, tested and protected in accordance with Approved Document H and pipe manufacturer requirements.

5.0 Surface Water Strategy

Roof water is collected via rainwater pipes and connected to the dedicated surface water drainage network. The upstream branches SW5-SW4 and SW3 combine at SW2. SW2 then connects directly to the existing on-site combined inspection chamber over an approximate length of 4.600 m. SW1 has been removed from the current layout.

A roof catchment of approximately 58 m² per roof has been used for branch sizing. Using a design rainfall intensity of 0.014 litres/second/m² gives a nominal runoff of 0.812 litres/second from each 58 m² roof. The adopted 100 mm pipework at 1:100 provides a conventional gravity drainage arrangement consistent with the stated design basis.

Driveway areas are to be permeable rather than directly connected as conventional impermeable paving wherever practicable. This reduces rapid runoff to the piped network and provides local source control. At boundaries where surface water could otherwise reach a footpath/highway, an interceptor channel or equivalent positive containment detail is to be provided where necessary.

6.0 Surface Water Design Calculations and Levels

The surface water system is designed back from the existing combined inspection chamber IL 165.100 m AOD using 100 mm nominal pipework at a minimum gradient of 1:100. Upstream invert = downstream invert + (run length / 100). Set-out levels are rounded upward to the nearest millimetre so that the constructed gradient is not flatter than the design minimum.

Run	Length (m)	Design gradient	Rise (m)	Downstream IL	Upstream IL
Existing IC - SW2	4.600	1:100	0.046	165.100	165.146
SW2 - SW3	10.475	1:100	0.105	165.146	165.251
SW2 - SW4	9.110	1:100	0.092	165.146	165.238
SW4 - SW5	5.200	1:100	0.052	165.238	165.290

Point	Cover Level (m AOD)	Invert Level (m AOD)	Chamber Depth (m)	Cover above pipe crown (m)
Existing combined IC	165.780	165.100	0.680	0.580
SW2	165.850	165.146	0.704	0.604
SW3	165.850	165.251	0.599	0.499
SW4	165.450	165.238	0.212	0.112
SW5	165.450	165.290	0.160	0.060

SW4 and SW5 remain relatively shallow. Appropriate pipe protection shall be provided to suit the final pavement construction, imposed loading and pipe manufacturer requirements.

7.0 Foul Drainage Strategy

Foul drainage from the dwellings is collected through inspection chambers FW3 and FW5. The FW5 branch passes via FW4 and combines with the FW3 branch at FW2. FW2 then connects directly to the existing on-site combined inspection chamber over an approximate length of 4.600 m. FW1 has been removed from the current layout.

The previously confirmed design loading comprises 5 WCs on the FW3 branch and 7 WCs on the FW5 branch, giving 12 WCs on the common downstream section at FW2. The shared foul system is designed as 150 mm nominal pipework at a minimum gradient of 1:150.

8.0 Foul Drainage Design Calculations and Levels

The foul drainage system is designed back from the existing combined inspection chamber IL 165.100 m AOD. Shared foul runs use 150 mm nominal pipework at a minimum gradient of 1:150. Upstream invert = downstream invert + (run length / 150), with set-out levels rounded upward to the nearest millimetre where necessary.

Run	Length (m)	Design gradient	Rise (m)	Downstream IL	Upstream IL
Existing IC - FW2	4.600	1:150	0.031	165.100	165.131
FW2 - FW3	6.425	1:150	0.043	165.131	165.174
FW2 - FW4	9.150	1:150	0.061	165.131	165.192
FW4 - FW5	6.973	1:150	0.047	165.192	165.239

Point	Cover Level (m AOD)	Invert Level (m AOD)	Chamber Depth (m)	Cover above pipe crown (m)
Existing combined IC	165.780	165.100	0.680	0.530
FW2	165.850	165.131	0.719	0.569
FW3	165.850	165.174	0.676	0.526
FW4	165.450	165.192	0.258	0.108

Point	Cover Level (m AOD)	Invert Level (m AOD)	Chamber Depth (m)	Cover above pipe crown (m)
FW5	165.450	165.239	0.211	0.061

FW4 and FW5 are shallow sections and shall receive suitable structural protection/detailing appropriate to the final loading and pipe system.

9.0 Hardstanding Reduction and Runoff Comparison

The redevelopment reduces hardstanding from approximately 520 m² to 420 m², a reduction of 100 m² or approximately 19.2%.

Scenario	Hardstanding Area (m ²)	Rainfall Depth (m)	Runoff Volume (m ³)
Existing	520	0.050	26.0
Proposed	420	0.050	21.0
Reduction	100	-	5.0

For a 50 mm rainfall depth, the calculated runoff volume from the hardstanding reduces from approximately 26.0 m³ to 21.0 m³, a reduction of approximately 5.0 m³ before allowing for the additional source-control benefit of permeable pavement storage and incidental infiltration/retention within the pavement build-up.

10.0 Permeable Driveway Construction

Driveways are to be formed as permeable pavements. A typical build-up shall comprise a permeable wearing course or permeable block paving, permeable laying course and approximately 200 mm of open-graded Type 3 gritstone/sub-base with interconnected voids. The material is to be free from excessive fines so that the permeable function is retained.

Where the driveway construction lies within a tree root protection area, the arboricultural method statement takes precedence and the cellular confinement/geotextile/open-graded aggregate build-up specified within that document is to be followed.

Conventional soakaways are not proposed because the superficial ground is of low permeability and has not been relied upon for full infiltration disposal. The permeable pavement is therefore treated as a source-control/attenuation feature rather than a soakaway. It will temporarily store water within the open-graded layer, reduce immediate surface runoff and permit incidental infiltration where local conditions allow.

Where driveway falls could direct residual surface water toward a public footway or highway, a linear interceptor/channel drain or equivalent positive containment measure shall be installed at the boundary and connected to a suitable on-site drainage route where gravity levels permit.

11.0 Exceedance, Protection and Maintenance

- External levels shall be formed to direct exceedance flows away from dwelling thresholds and toward landscaped or controlled drainage areas.
- No surface water shall be intentionally discharged onto adjoining land or the public highway.
- Drainage pipes with reduced cover shall be protected in accordance with pipe manufacturer requirements and Approved Document H, using suitable bedding, structural protection slab or encasement where necessary.
- Inspection chambers, gullies, rainwater outlets and channel drains shall remain accessible for inspection and maintenance.
- Permeable paving shall be maintained so that the surface remains free draining; accumulated silt and debris shall be removed periodically and damaged/jointed areas repaired.
- Property owners shall retain responsibility for maintenance of private drainage and permeable pavement serving their respective plots unless formally adopted by another body.

12.0 Construction and Verification

- All drainage levels shall be checked on site against the latest setting-out and finished level information before excavation.
- The existing on-site combined inspection chamber cover level 165.780 m AOD and invert level 165.100 m AOD shall be verified before construction of the upstream drainage.
- SW2 and FW2 shall be independently connected to the existing combined inspection chamber in accordance with the final drainage drawing.
- Pipes shall be laid to the scheduled gradients on suitable bedding and backfill appropriate to pipe material, depth and loading.
- All drains shall be tested prior to covering and chamber benching/channels formed to provide smooth hydraulic transitions.
- Any discrepancy between surveyed levels and this strategy shall be referred back for review before the affected drainage run is installed.
- Final as-built invert and cover levels should be recorded for the drainage completion file.

13.0 Conclusion

The revised drainage strategy provides separate foul and surface water site systems with both systems discharging to the existing on-site combined inspection chamber at IL 165.100 m AOD. SW1 and FW1 have been removed; SW2 and FW2 are now the final proposed chambers and each connects directly to the existing combined chamber over approximately 4.600 m.

The foul system uses 150 mm shared drainage at a minimum 1:150 gradient and the surface water system uses 100 mm drainage at a minimum 1:100 gradient. The revised chamber cover and invert levels provide continuous gravity falls to the existing chamber. Shallow upstream sections at SW4/SW5 and FW4/FW5 are to receive appropriate protection to suit the final construction and loading.

The proposal also reduces hardstanding by approximately 19.2%, giving a 5.0 m³ reduction in theoretical runoff volume for a 50 mm rainfall event. Permeable driveways provide additional source-control storage and runoff reduction. They are not relied upon as conventional soakaways and do not conflict with the conclusion that soakaway disposal is unsuitable for the low-permeability site.

Subject to verification of the existing combined chamber level and connection before construction, installation to the scheduled gradients and levels, and appropriate maintenance/protection of the drainage and permeable paving, the strategy provides a coherent basis for discharge of the drainage planning condition.

14.0 References

- HM Government - Approved Document H: Drainage and Waste Disposal (current GOV.UK edition).
- Department for Environment, Food & Rural Affairs - National standards for sustainable drainage systems (SuDS), 2025.
- Phase 1 Environmental Desk Study - Former Ambulance Station, Cemetery Road, Ramsbottom.
- Arboricultural Method Statement - Cemetery Road.
- Proposed Site Drainage drawing 25.372 C.12A - current revision.

Surface Water Point	Run	Length	Gradient	Rise	Invert Level	Cover Level	Chamber Depth	
Existing combined IC	—	—	—	—	165.100	165.780	0.680	
SW2	IC → SW2	4.600 m	1:100	0.046	165.146	165.850	0.704	
SW3	SW2 → SW3	10.475 m	1:100	0.105	165.251	165.850	0.599	
SW4	SW2 → SW4	9.110 m	1:100	0.091	165.237	165.450	0.213	
SW5	SW4 → SW5	5.200 m	1:100	0.052	165.289	165.450	0.161	

Foul Point	Run	Length	Gradient	Rise	Invert Level	Cover Level	Chamber Depth	
Existing combined IC	—	—	—	—	165.100	165.780	0.680	
FW2	IC → FW2	4.600 m	1:150	0.031	165.131	165.850	0.719	
FW3	FW2 → FW3	6.425 m	1:150	0.043	165.174	165.850	0.676	
FW4	FW2 → FW4	9.150 m	1:150	0.061	165.192	165.450	0.258	
FW5	FW4 → FW5	6.973 m	1:150	0.047	165.239	165.450	0.211	

Notes

THIS DRAWING IS TO BE READ IN CONJUNCTION WITH ALL RELEVANT HDB, ARCHITECTS, SERVICE ENGINEERS AND SUBCONTRACTOR DRAWINGS.

REVIEW ALL DRAWINGS AND REPORT ANY DISCREPANCIES TO HDB PRIOR TO COMMENCEMENT.

DO NOT SCALE FROM THIS DRAWING. ALL DIMENSIONS AND LEVELS INCLUDING ANY ABUTMENT TO EXISTING STRUCTURES TO BE CHECKED ON SITE PRIOR TO COMMENCEMENT.

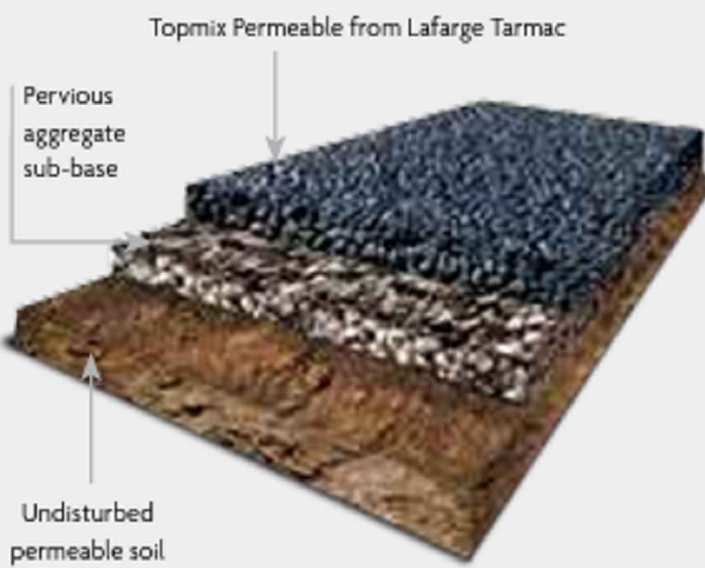
WORK FROM FIGURED DIMENSIONS ONLY. DO NOT SCALE FROM THIS DRAWING.

ALL DIMENSIONS IN MILLIMETRES AND LEVELS IN METRES, TO BE CHECKED ON SITE

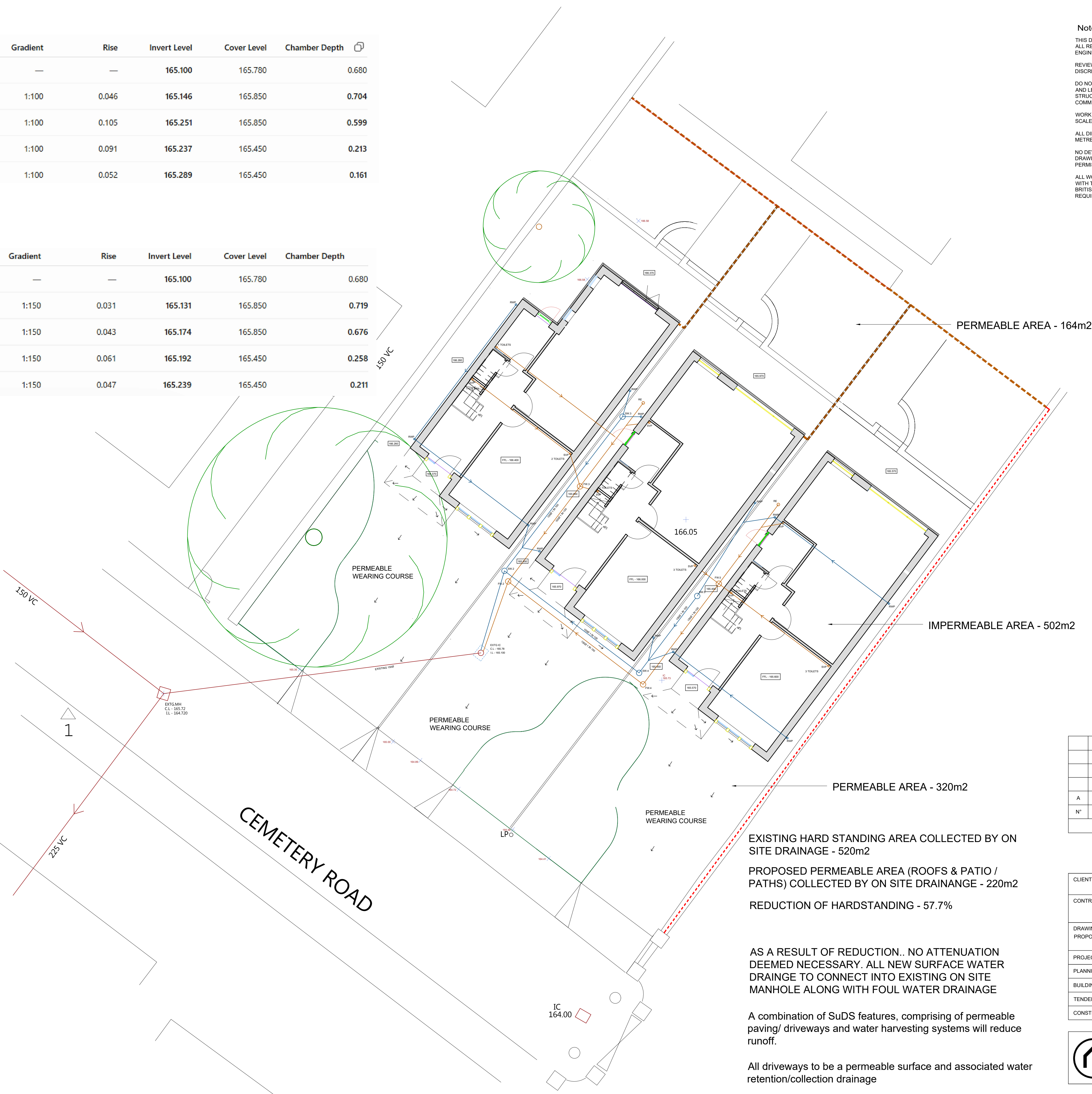
NO DEVIATION FROM DETAILS SHOWN ON THIS DRAWING IS ALLOWED WITHOUT HDB PRIOR PERMISSION IN WRITING.

ALL WORK IS TO BE CARRIED OUT IN ACCORDANCE WITH THE RELEVANT SPECIFICATIONS ISSUED BY HDB, BRITISH STANDARD CODES OF PRACTICE, STATUTORY REQUIREMENTS AND THE CONTRACT DOCUMENTS.

SYSTEM A – Full infiltration



This system allows all water falling onto the pavement to permeate through the Topmix Permeable surface layer, pass through the lower pavement courses and into the sub-grade. Some water may be retained within the pavement reservoir before permeating into the sub-grade. Full infiltration systems do not discharge any additional water into traditional drainage systems. Geotextiles may be incorporated into the system depending on the nature of the sub-grade.



A	25.04.24	ISSUED FOR PLANNING
N°	DATE	DESCRIPTION
REVISION		

CLIENT:					
CONTRACT:					
DRAWING TITLE: PROPOSED SITE DRAINAGE					
PROJECT N°:	25.372	Drq N°:	C.12A	Rev:	A
PLANNING:		SCALE:			1/100
BUILDING REGS:		DATE:			25.04.24
TENDER:		DRAWN:			AG
CONSTRUCTION:		CHECKED:			



EXISTING HARD STANDING AREA COLLECTED BY ON SITE DRAINAGE - 520m2

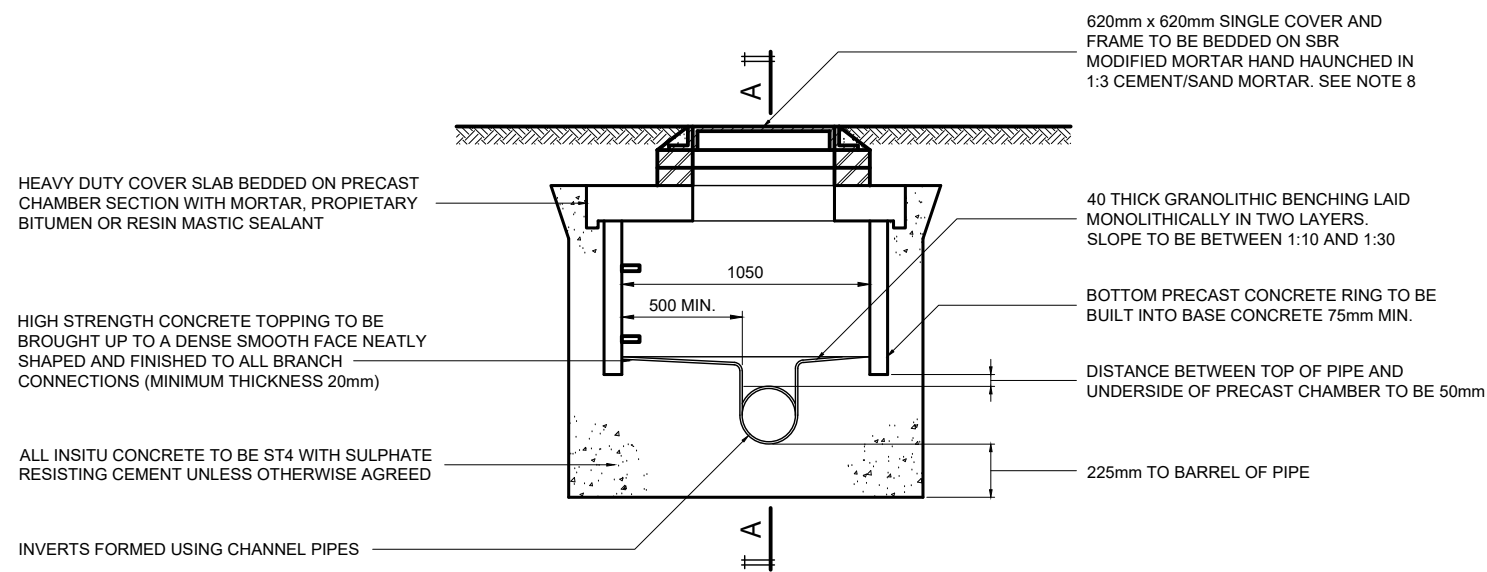
PROPOSED PERMEABLE AREA (ROOFS & PATIO / PATHS) COLLECTED BY ON SITE DRAINAGE - 220m2

REDUCTION OF HARDSTANDING - 57.7%

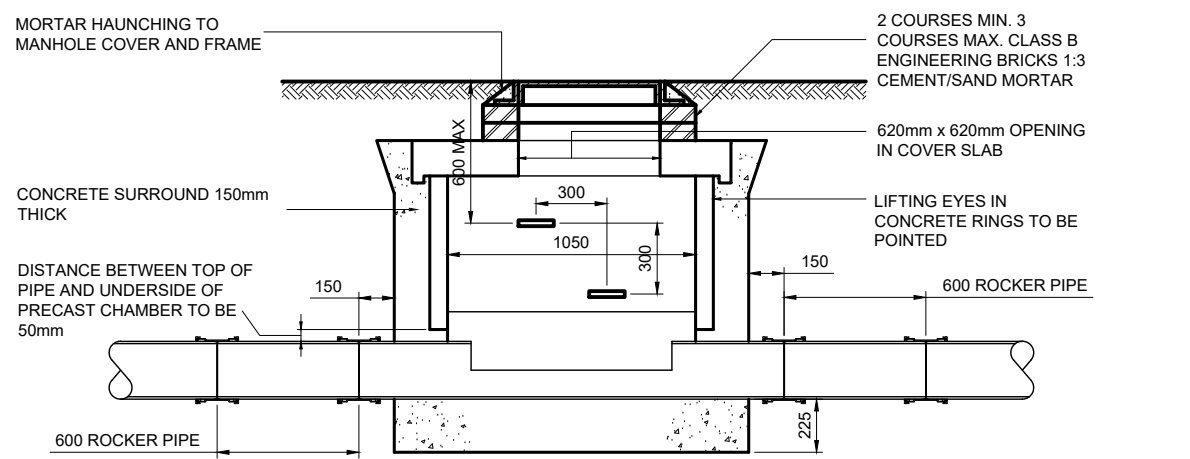
AS A RESULT OF REDUCTION.. NO ATTENUATION DEEMED NECESSARY. ALL NEW SURFACE WATER DRAINAGE TO CONNECT INTO EXISTING ON SITE MANHOLE ALONG WITH FOUL WATER DRAINAGE

A combination of SuDS features, comprising of permeable paving/ driveways and water harvesting systems will reduce runoff.

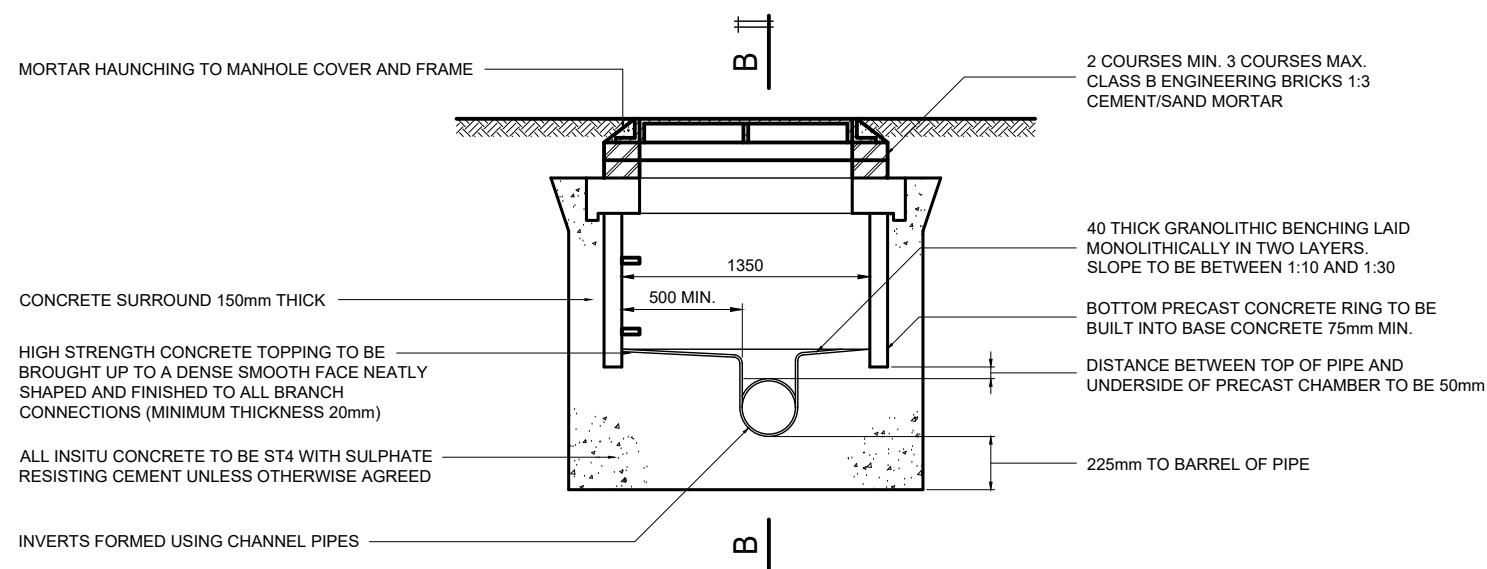
All driveways to be a permeable surface and associated water retention/collection drainage



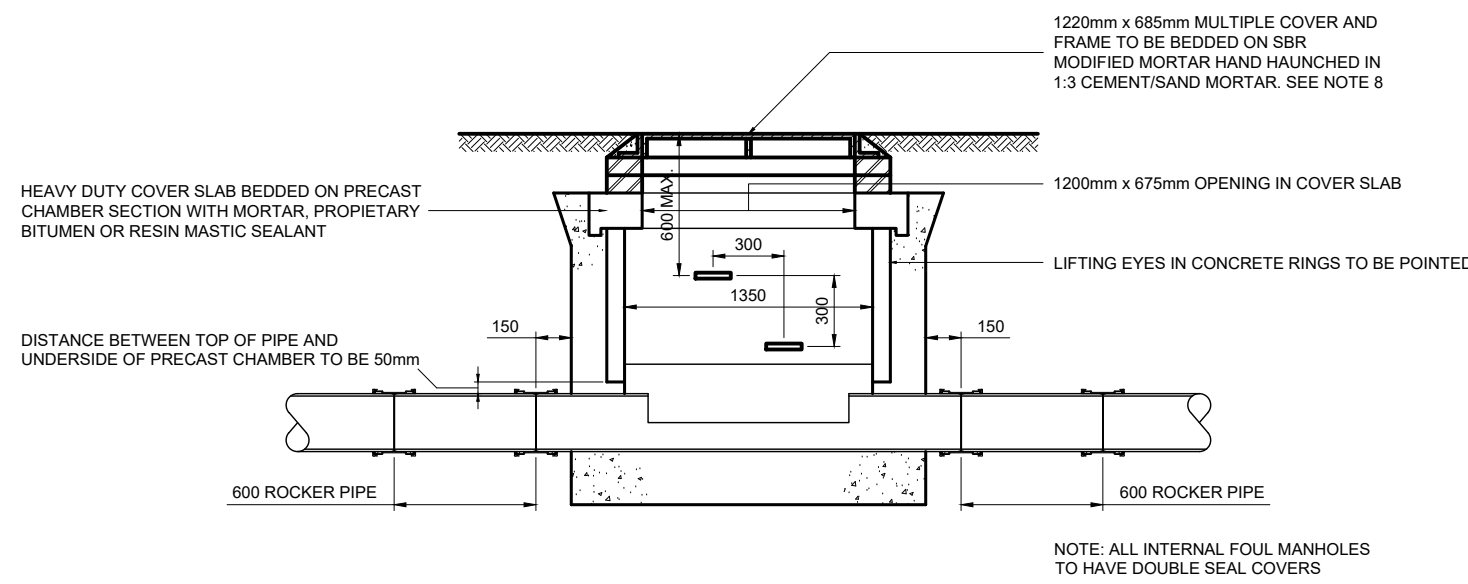
TYPICAL MANHOLE DETAIL DEPTH FROM COVER
LEVEL TO SOFFIT OF PIPE LESS THAN 1.0m



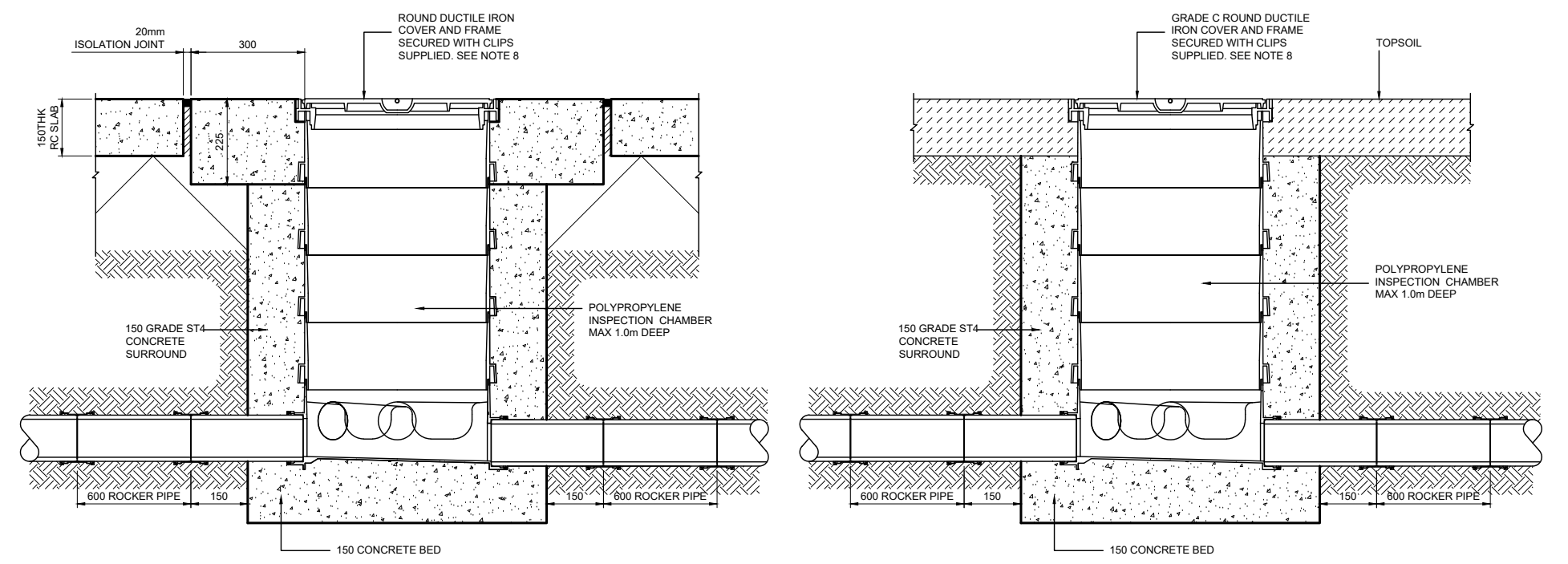
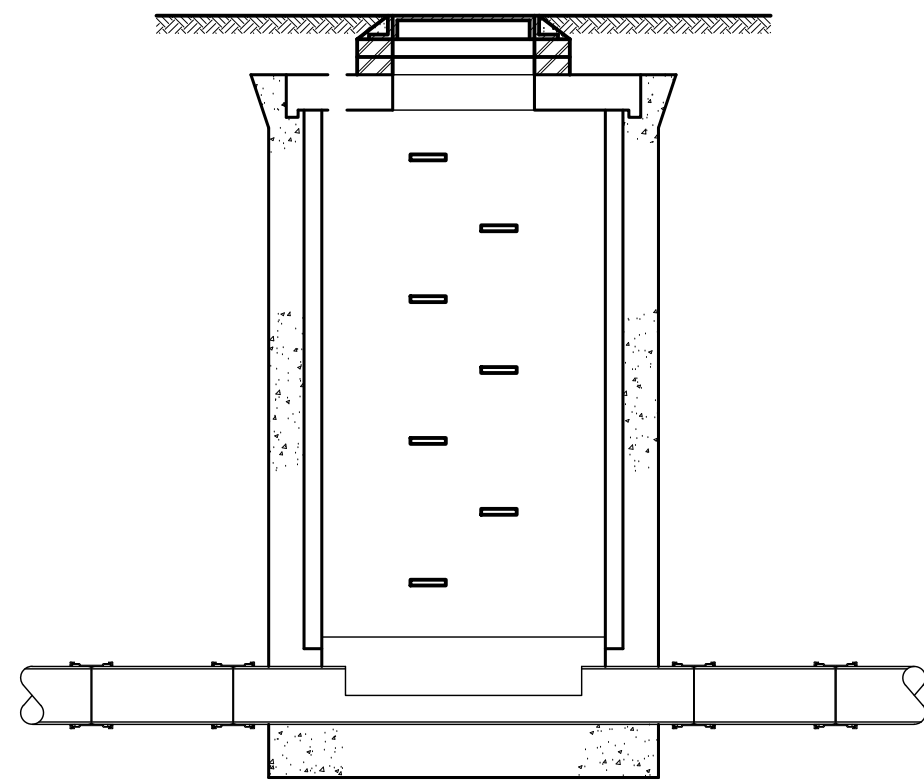
TYPICAL MANHOLE DETAIL DEPTH FROM COVER
LEVEL TO SOFFIT OF PIPE 1.0m TO 1.5m



TYPICAL MANHOLE DETAIL DEPTH FROM
COVER LEVEL TO SOFFIT OF PIPE 1.5m TO 3.0m



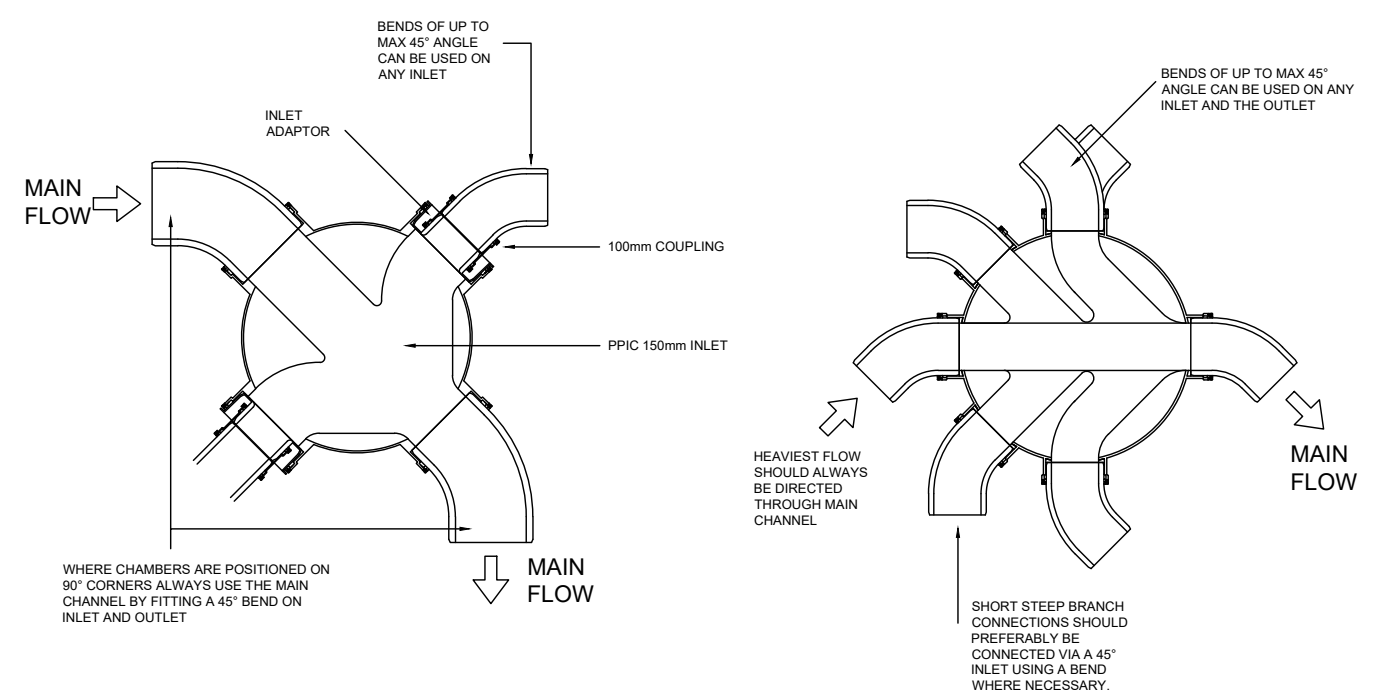
SECTION C-C



SITED IN EXTERNAL ROAD SLABS

SITED IN SOFT LANDSCAPED OR PAVED AREAS

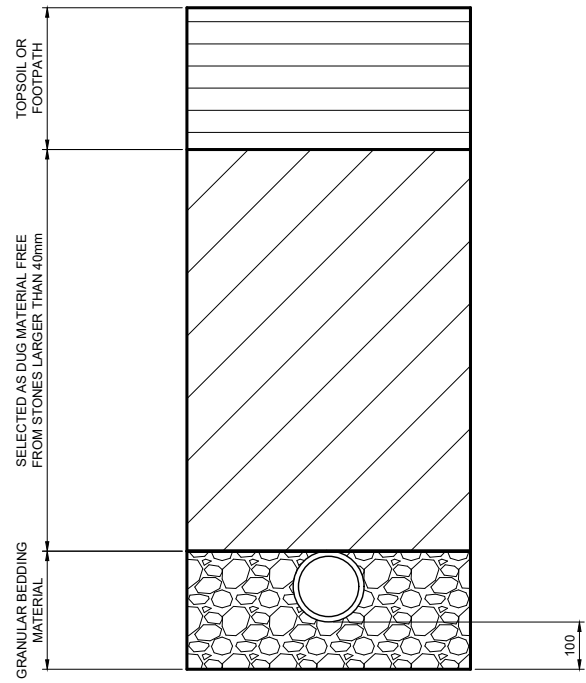
INSPECTION CHAMBER
(PIPE DIAMETER 100Ø AND 150Ø)
(MAXIMUM DEPTH TO INVERT 600-1000mm)



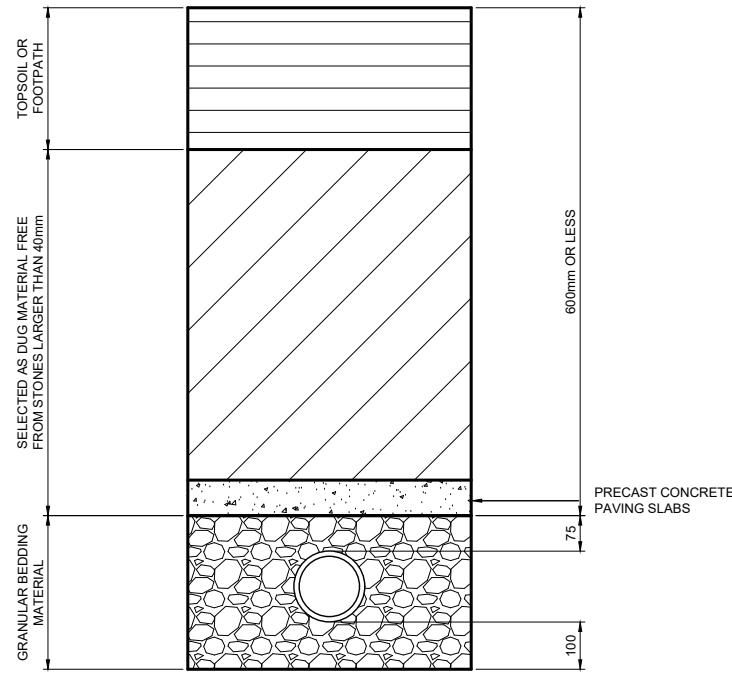
INSPECTION CHAMBER BASE ARRANGEMENTS

N°	DATE	DESCRIPTION
REVISION		

CLIENT:				
CONTRACT: CEMETERY ROAD				
DRAWING TITLE: PROPOSED DRAINAGE				
PROJECT N°:		Drig N°:	C.12B	Rev:
PLANNING:		SCALE:	1/50	
BUILDING REGS:		DATE:	13.09.23	
TENDER:		DRAWN:	SC	
CONSTRUCTION:		CHECKED:		



SOFT LANDSCAPED AREAS
COVER TO PIPE GREATER THAN 600mm

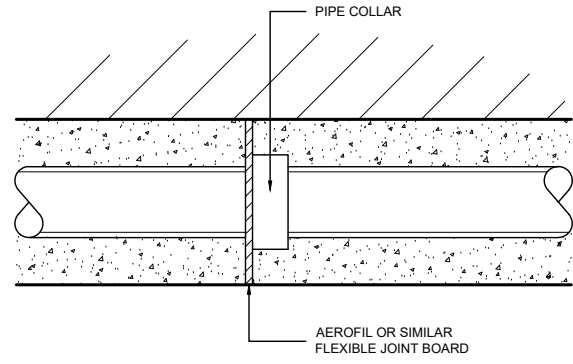


SOFT LANDSCAPED AREAS
COVER TO PIPE LESS THAN 600mm

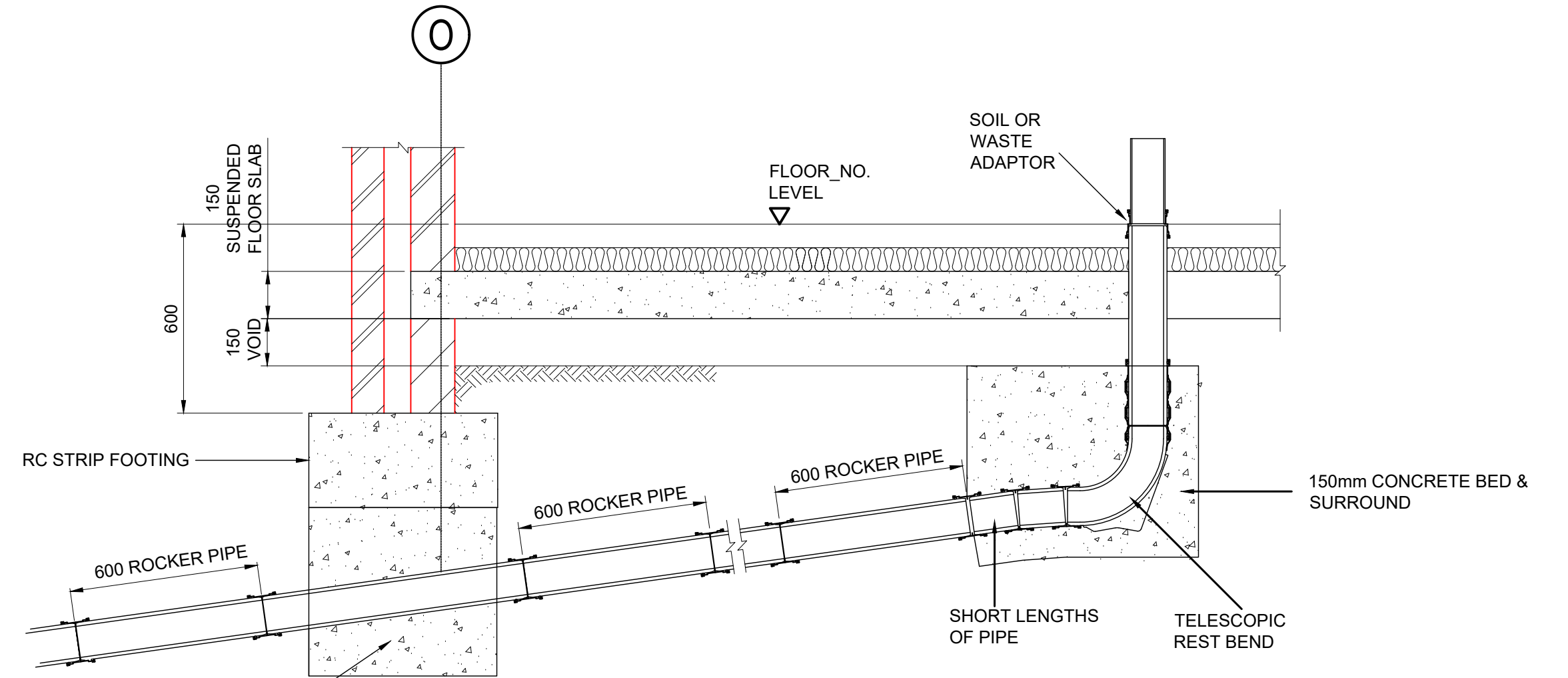
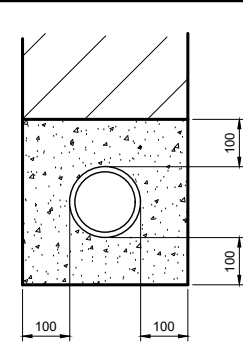
Pipe Ø	Trench Width	Uniform Soil (A)	Rock (A)
100	550	100	200
150	600	100	200
225	700	150	200
300	750	150	200
375	1050	150	200
450	1150	150	200

Sizing Of Bedding Material		
Nominal Bore Of Pipe, mm	Size, mm Single Sized	Size, mm Graded
100-200	10 or 14	14 to 5
225-300	10, 14 or 20	14 to 5 or 20 to 5
375-600	14 or 20	14 to 5 or 20 to 5
600+	14, 20 or 40	14 to 5, 20 to 5 or 40 to 5

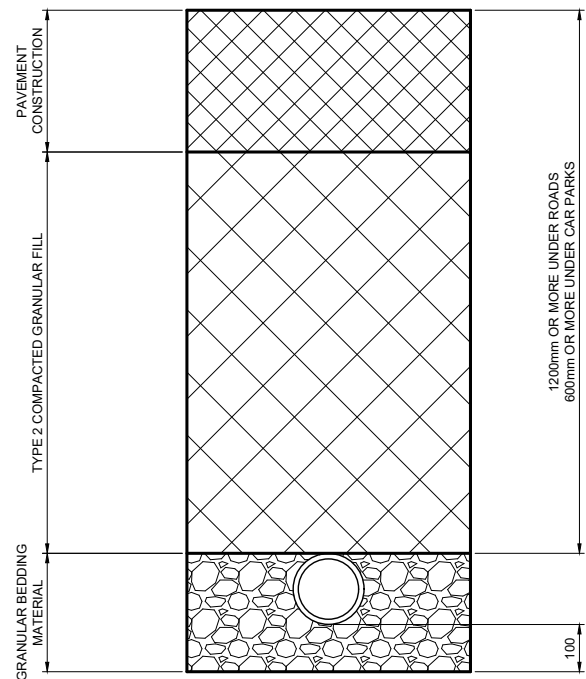
All granular material to be single sized or graded in accordance with BS882 1992, unless otherwise stated. BS3757 1990 and its revised third edition 1993 are suitable.



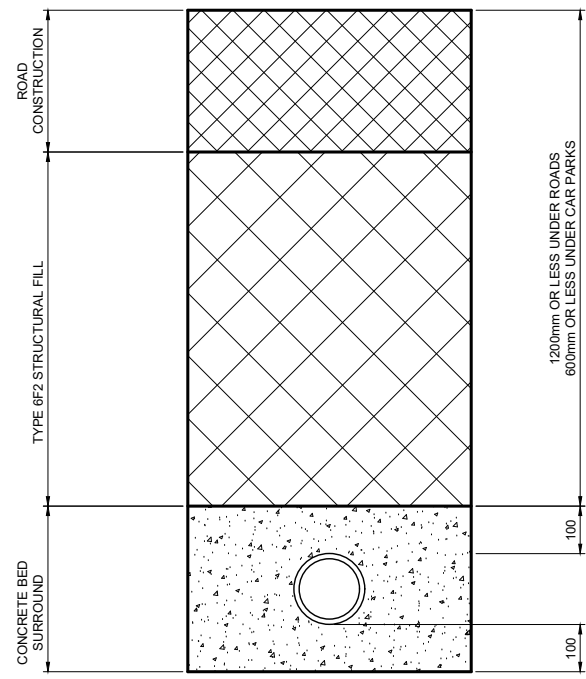
MINIMUM CONCRETE ENCASEMENT FOR RIGID PIPES SHOWING FLEXIBLE BOARD



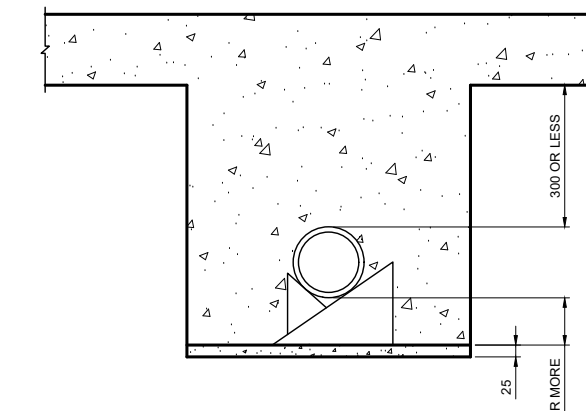
DRAIN & FOUNDATION DETAIL
SCALE 1:20



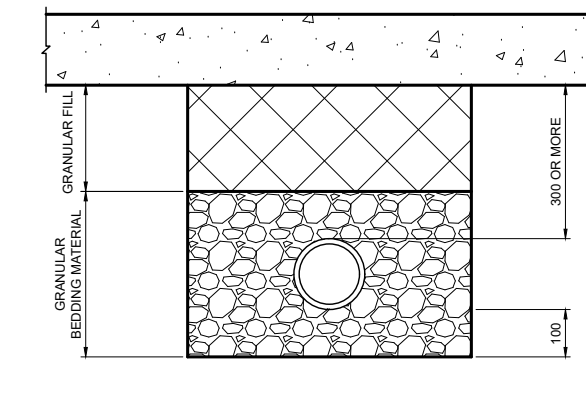
UNDER LIGHTLY TRAFFICKED ROADS



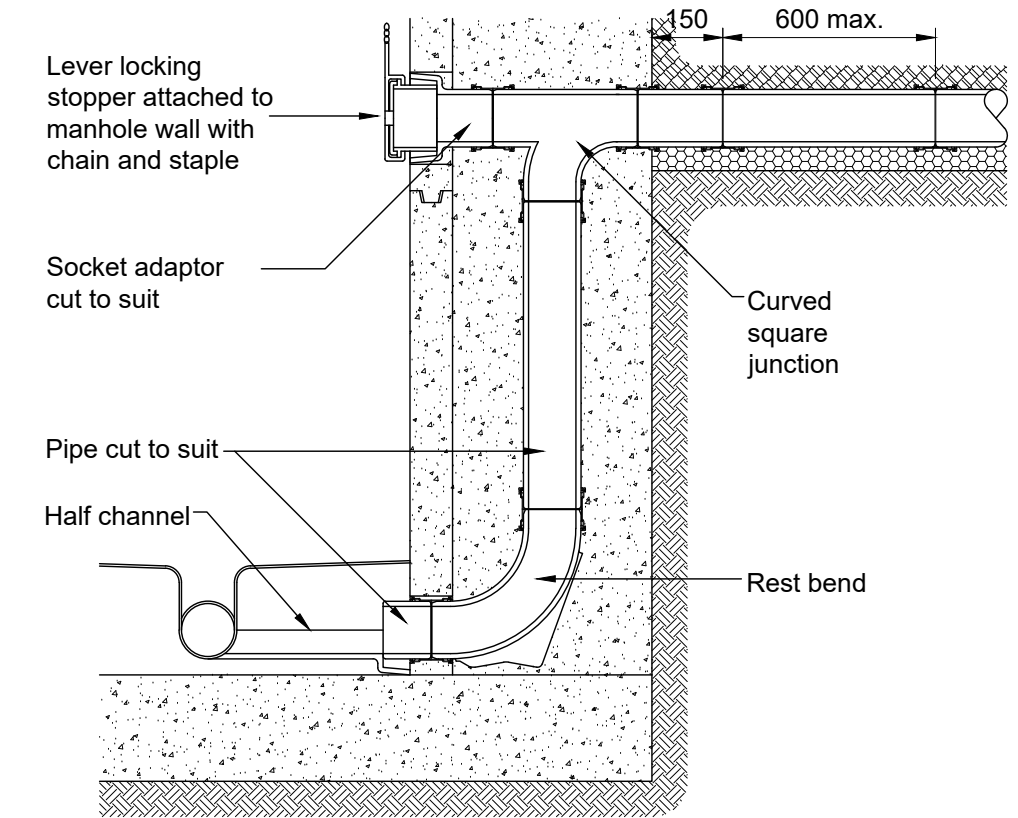
UNDER LIGHTLY TRAFFICKED
ROADS PIPES AT SHALLOW DEPTH



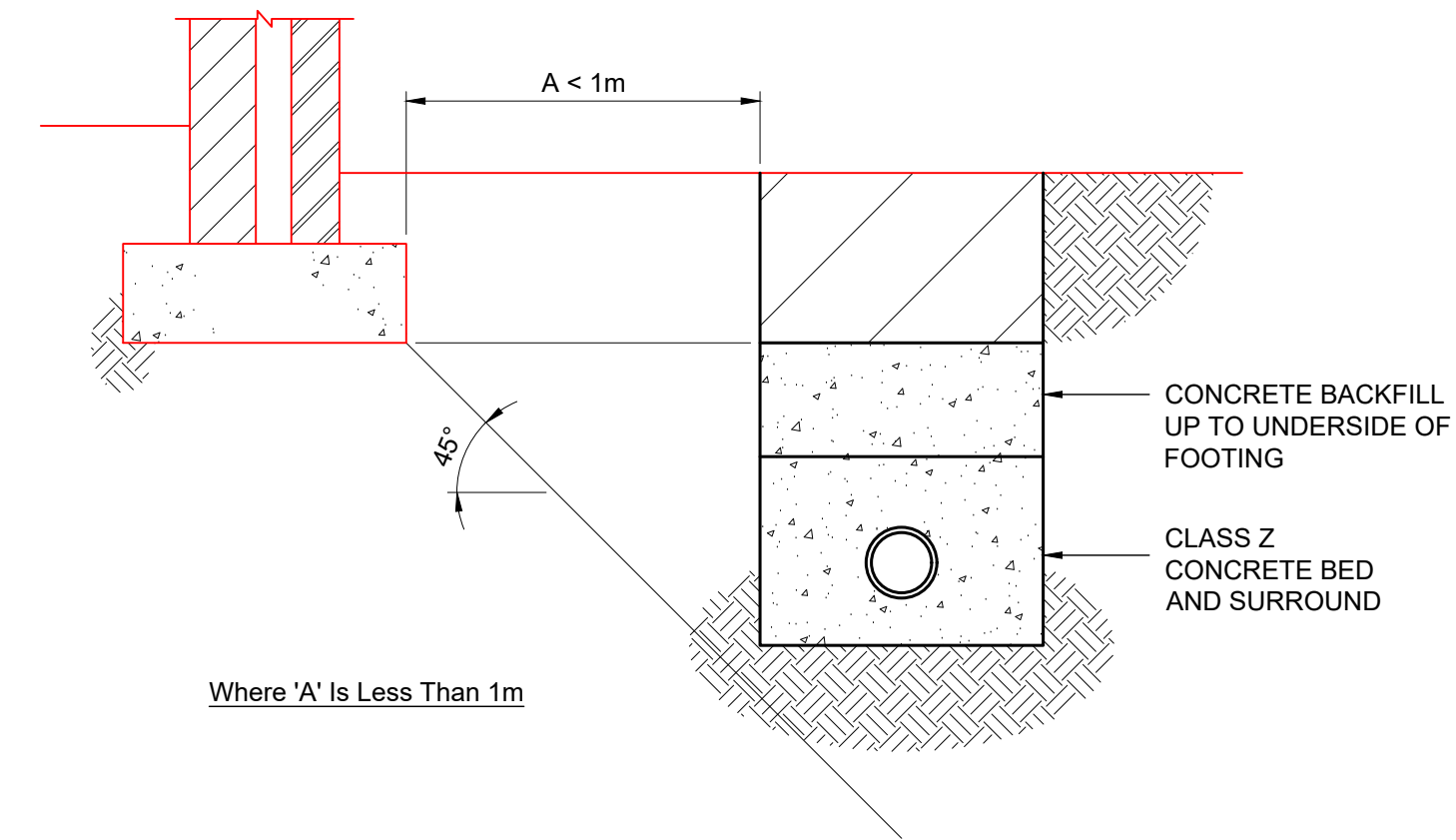
CONCRETE SURROUND FOR SHALLOW PIPES UNDER
BUILDINGS DEPTH TO PIPE LESS THAN 300mm



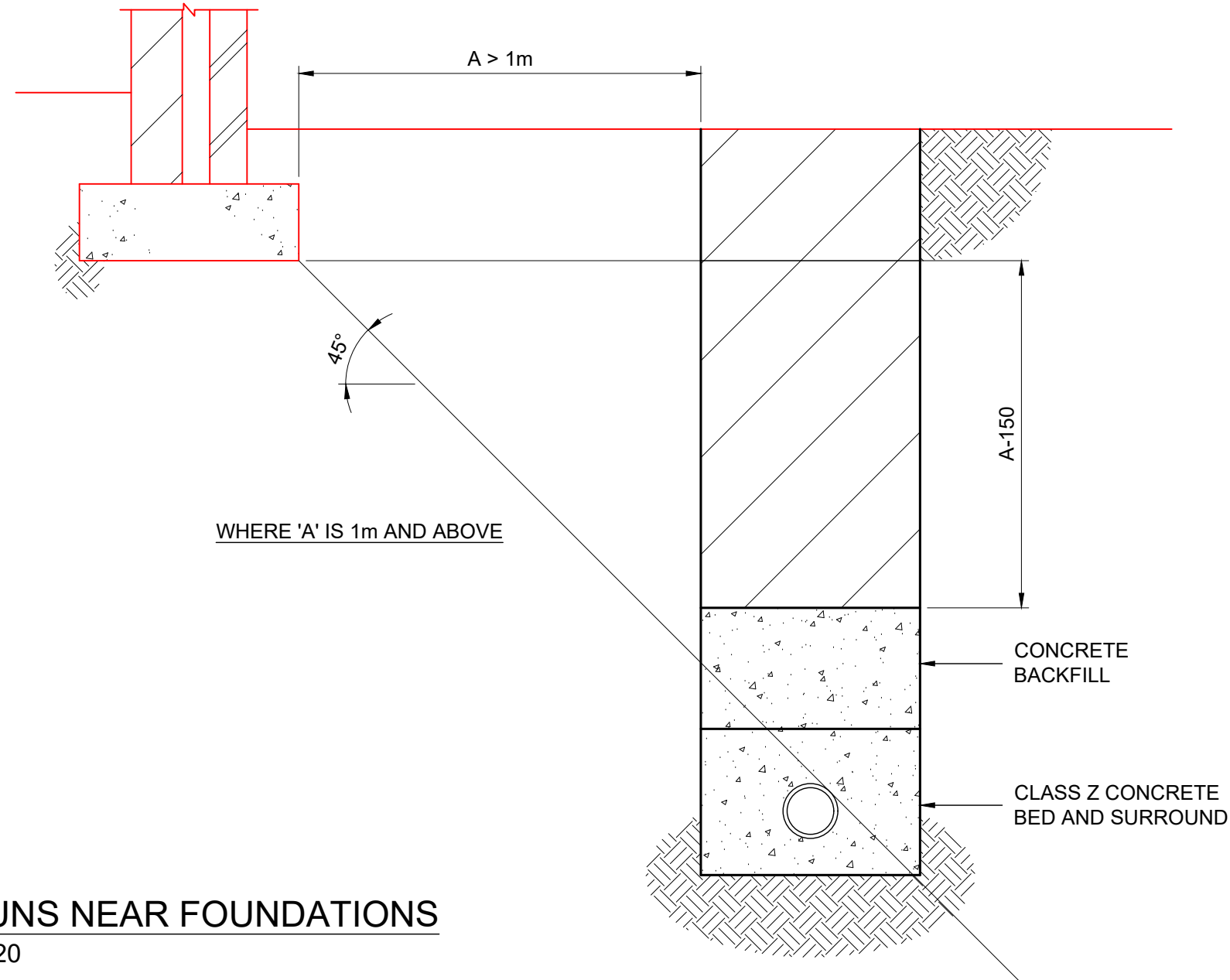
BEDDING FOR PIPES UNDER SLABS
COVER TO PIPE GREATER THAN 300mm



BACKDROP CONNECTION TO MANHOLE
SCALE 1:20



Where 'A' is Less Than 1m



WHERE 'A' IS 1m AND ABOVE

TYPICAL DETAILS FOR PIPE RUNS NEAR FOUNDATIONS
SCALE 1:20

N°	DATE	DESCRIPTION
REVISION		

CLIENT:			
CONTRACT: CEMETERY ROAD			
DRAWING TITLE: PROPOSED DRAINAGE			
PROJECT N°:		Dwg N°:	C.12C Rev:
PLANNING:		SCALE:	1/50
BUILDING REGS:		DATE:	13.09.23
TENDER:		DRAWN:	SC
CONSTRUCTION:		CHECKED:	